

Coal Combustion Residue Impoundment

Round 9 - Dam Assessment Report

Oklaunion Power Station

Wastewater Evaporation Pond 6 – WMU 001 American Electric Power Vernon, Texas

Prepared for:

United States Environmental Protection Agency Office of Resource Conservation and Recovery

Prepared by:

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INTRODUCTION, SUMMARY CONCLUSIONS AND RECOMMENDATIONS

The release of over five million cubic yards of coal combustion waste from the Tennessee Valley Authority's Kingston, Tennessee facility in December 2008 flooded more than 300 acres of land, damaging homes and property. In response the U.S. EPA is assessing the stability and functionality of coal combustion ash impoundments and other management units across the country and, as necessary, identifying any needed corrective measures.

This assessment of the stability and functionality of the Oklaunion Power Station Wastewater Evaporation Ponds is based on a review of available documents and on the site assessment conducted by Dewberry personnel on Wednesday, February 23, 2011. We found the supporting technical documentation adequate (Section 1.1.3). As detailed in Section 1.2, there are five (5) recommendations based on field observations that may help to maintain a safe and trouble-free operation.

In summary, the Oklaunion Power Station Wastewater Evaporation Ponds are rated **POOR** for continued safe and reliable operation. These ratings are based only on the lack of critical studies and investigations available to the assessors to determine the inundation potential of the dams and potential for dam safety deficiencies or the future closure plans for the non-incised ponds. If the non-incised pond (Pond 6) is to remain open, the following recent and current information, studies and analysis are needed: breach analysis, and hydraulic and hydrological studies. If the non-incised pond (Pond 6) is in the process of being closed, a report indicating the closure plan and schedule needs to be submitted. Upon receipt of data showing adequate hydraulic and structural soundness or a proper closure plan the rating can be changed to satisfactory.

PURPOSE AND SCOPE

The U.S. Environmental Protection Agency (EPA) is embarking on an initiative to investigate the potential for catastrophic failure of Coal Combustion Surface Impoundments (i.e., management unit) from occurring at electric utilities in an effort to protect lives and property from the consequences of a dam failure or the improper release of impounded slurry. The EPA initiative is intended to identify conditions that may adversely affect the structural stability and functionality of a management unit and its appurtenant structures (if present); to note the extent of deterioration (if present), status of maintenance and/or a need for immediate repair; to evaluate conformity with current design and construction practices; and to determine the hazard potential classification for units not currently classified by the management unit owner or by a state or federal agency. The initiative will address management units that are classified as having a Less-than-Low, Low, Significant or High Hazard Potential ranking. (For Classification, see pp. 3-8 of the 2004 Federal Guidelines for Dam Safety)

In early 2009, the EPA sent its first wave of letters to coal-fired electric utilities seeking information on the safety of surface impoundments and similar facilities that receive liquid-borne material that store or dispose of coal combustion residue. This letter was issued under the authority of the Comprehensive Environmental Response, Compensation, and Liability Act (CERCLA) Section 104(e), to assist the Agency in assessing the structural stability and functionality of such management units, including which facilities should be visited to perform a safety assessment of the berms, dikes, and dams used in the construction of these impoundments.

EPA requested that utility companies identify all management units including surface impoundments or similar diked or bermed management units or management units designated as landfills that receive liquid-borne material used for the storage or disposal of residuals or byproducts from the combustion of coal, including, but not limited to, fly ash, bottom ash, boiler slag, or flue gas emission control residuals. Utility companies provided information on the size, design, age and the amount of material placed in the units. The EPA used the information received from the utilities to determine preliminarily which management units had or potentially could have High Hazard Potential ranking.

The purpose of this report is **to evaluate the condition and potential of residue release from management units for hazard potential classification**. This evaluation included a site visit. Prior to conducting the site visit, a two-person team reviewed the information submitted to EPA, reviewed any relevant publicly available information from state or federal agencies regarding the unit hazard potential classification (if any) and accepted information provided via telephone communication with the management unit owner. Also after the February 23, 2011 field visit, additional information was received on March 22, 2011, by Dewberry & Davis LLC about the Oklaunion Power Station Wastewater Evaporation Ponds that was reviewed and used in preparation of this report.

Factors considered in determining the hazard potential classification of the management units(s) included the age and size of the impoundment, the quantity of coal combustion residuals or by-products that were stored or disposed of in these impoundments, its past operating history, and its geographic location relative to down gradient population centers and/or sensitive environmental systems.

This report presents the opinion of the assessment team as to the potential of catastrophic failure and reports on the condition of the management unit(s).

Oklaunion Power Station American Electric Power Vernon, Texas

LIMITATIONS

The assessment of dam safety reported herein is based on field observations and review of readily available information provided by the owner/operator of the subject coal combustion residue management unit(s). Qualified Dewberry engineering personnel performed the field observations and review and made the assessment in conformance with the required scope of work and in accordance with reasonable and acceptable engineering practices. No other warranty, either written or implied, is made with regard to our assessment of dam safety.

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APPENDIX A

Doc 01:	Aerial Photo
Doc 02:	Plans Original Site & Boring Locations
Doc 03:	Plans Pond Details 1
Doc 04:	Original Specifications
Doc 05:	Inspection Report 06.15.09
Doc 06:	Inspection Report 8.04.10
Doc 07:	TCEQ 2009 Inspection Report
Doc 08:	TCEQ 2010 Inspection Report.

APPENDIX B

Doc 9:	Coal Combustion Dam Inspection Check	list Form
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APPENDIX C

Doc 10:	additional	l site photograph	IS
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1.0 CONCLUSIONS AND RECOMMENDATIONS

1.1 CONCLUSIONS

Conclusions are based on visual observations from a one-day site visit, February 23, 2011, and review of technical documentation provided by American Electric Power.

1.1.1 Conclusions Regarding the Structural Soundness of the Management Unit(s)

The dike embankments appeared to be structurally sound based on Dewberry engineers' observations during the site visit. However, No geotechnical analyses were provided to Dewberry. Therefore, no determination can be made regarding Structural Soundness of the Management Unit.

1.1.2 Conclusions Regarding the Hydrologic/Hydraulic Safety of the Management Unit(s)

No hydrologic or hydraulic analyses were provided to Dewberry. Therefore, no determination can be made regarding Hydrologic/Hydraulic Safety of the Management Unit.

1.1.3 Conclusions Regarding the Adequacy of Supporting Technical Documentation

The supporting technical documentation was inadequate with the exception of original Plans and Specifications of the of the Management Unit. Engineering documentation provided and subsequently reviewed is included in Appendix A.

1.1.4 Conclusions Regarding the Description of the Management Unit(s)

The description of the management unit provided by the Owner was a fairly accurate representation of what Dewberry observed in the field. Pond 6 is the only non-incised CCR pond that has been in service in the recent past. All other ponds on site either are incised or do not receive CCR. Also Pond 6 is in the process of being filled and eventually will be closed.

1.1.5 Conclusions Regarding the Field Observations

Dewberry staff was provided access to all areas in the vicinity of the management unit required to conduct a thorough field observation. The visible parts of the embankment dikes observed had no signs of overstress,

significant settlement, shear failure, or other signs of instability. Embankments appeared structurally sound. There were no apparent indications of unsafe conditions or conditions needing remedial action. However, the only non incised pond (Pond 6) that received CCR appeared to be in the process of being filled and closed.

1.1.6 Conclusions Regarding the Adequacy of Maintenance and Methods of Operation

The current maintenance and methods of operation appear to be adequate for the fly ash management unit. There was no evidence of significant embankment repairs or prior releases observed during the field inspection. However, there is no Maintenance procedure documented.

1.1.7 Conclusions Regarding the Adequacy of the Surveillance and Monitoring Program

The surveillance program appears to be adequate. The management unit dikes are not instrumented. Based on a history of a current and regular inspection program and the fact that the only non-incised CCR pond (Pond 6) is in the process of being closed, installation of a dike monitoring system is not needed at this time. However, if Pond 6 is kept in operation a Surveillance and Monitoring Program should be documented.

1.1.8 Classification Regarding Suitability for Continued Safe and Reliable Operation

The facility is satisfactory with the exception of a Breach Analysis and Inundation Map or a closure plan and schedule for the non-incised CCR pond (Pond 6). Therefore, the Management Unit is rated POOR for continued safe and reliable operation until the receipt of the deficient documentation.

1.2 RECOMMENDATIONS

1.2.1 Recommendations Regarding the Structural Stability

We recommend AEP conduct a geotechnical analysis of the impoundment structure or provide a closure plan and schedule for Pond 6.

1.2.2 Recommendations Regarding the Hydrologic/Hydraulic Safety

We recommend AEP conduct a Hydraulic and Hydrologic study that includes a Breach Analysis and Inundation Map if Pond 6 is not closed in the near future

1.2.3 Recommendations Regarding the Supporting Technical Documentation

The following recommendation is warranted: if Pond 6 is to be closed the operator should provide a closure plan and schedule.

1.2.4 Recommendations Regarding the Maintenance and Methods of Operation

It is recommended that AEP develop and document a Maintenance and Methods of Operation Procedure, including checklists, or, as stated in Section 1.2.3, the plant should provide a closure plan and schedule for Pond 6.

1.2.5 Recommendations Regarding Continued Safe and Reliable Operation

The recommendations cited above, upon implementation, will ensure continued safe and reliable operation.

1.3 PARTICIPANTS AND ACKNOWLEDGEMENT

1.3.1 List of Participants

David Scott, Oklaunion Power Station William R. Smith, American Electric Power Service Company Steve Lewis, American Electric Power Kyle Shepard, P.E., Dewberry Andrew Cueto, P.E. PMP, Dewberry

1.3.2 Acknowledgement and Signature

We acknowledge that the management unit referenced herein has been assessed on February 23, 2011.

Andrew Cueto, P.E., PMP

Kyle Shepard, P.E.

2.0 DESCRIPTION OF THE COAL COMBUSTION RESIDUE MANAGEMENT UNIT(S)

2.1 LOCATION AND GENERAL DESCRIPTION

The Oklaunion Power Station is located at 12567 FM Rd 3430, Vernon, TX 76384, approximately three miles south-southeast of the intersection of Farm to Market Road 433 and Farm to Market Road 3430 in Wilbarger County, Texas

The plant is operated by American Electric Power Service Corporation and is a coal fired facility which features 17 total wastewater evaporation ponds with a total area of 335.9 acres: six (6) of the wastewater ponds contain some byproducts of the coal combustion process while the 11other ponds contain cooling tower blowdown. The six ponds containing CCR are primarily located in the south central portion of the plant site.

Of the six CCR ponds five are incised; the sixth has a side hill embankment (Pond 6) with a height of 20 feet and a pool area of 58 acres. In past operations, earthen fill dikes retain the waste until it is sufficiently dry to be hauled away and landfilled. The Wastewater Evaporation Pond 6 is located at the south-central edge of the main evaporation pond complex of the generating station. An aerial photograph of the impoundment is provided in Appendix A – Doc. 01.

The Oklaunion Power Station Wastewater Evaporation Pond 6 was reported to be in the process of being filled with non-CCR material and is in the process of being closed. The impoundment was reported to not have received CCR for over three (3) years). The construction documents were issued on March 1, 1983 and construction was complete in 1987 as noted in Document 2 and 3 of Appendix A.

The table below provides the dimensions of the embankment:

Table 2.1: Summary of Dam Dimensions and Size						
Northeastern Station Bottom Ash Basin						
Dam Height (ft)	20 feet					
Crest Width (ft)	20 feet					
Length (ft)	3968 feet					
Side Slopes (upstream) H:V	N/A					
Side Slopes (downstream)						
H:V	3:1					

2.2 COAL COMBUSTION RESIDUE HANDLING

2.2.1 Fly Ash

The Fly Ash disposal process is a dry train procedure. AEP representatives preferred not to tour and inspect the Fly Ash disposal train directly. However, they did describe the process as follows:

- 1. Fly Ash is electrostatically precipitated and conveyed by gravity to a hopper,
- 2. The ash is then pneumatically conveyed into a holding silo,
- 3. The ash is then loaded via gravity feed into trucks (3rd party) to be transported to a landfill,
- 4. The ash is stockpiled to be either sold to a 3^{rd} party for beneficial reuse or to be permanently disposed of in the landfill.

2.2.2 Bottom Ash

The Bottom Ash disposal process is a wet train procedure. Bottom ash is removed from the boiler and sluiced to the wastewater ponds.

2.2.3 Boiler Slag

Boiler slag is same as Bottom Ash. See entry above.

2.2.4 Flue Gas Desulfurization Sludge

Flue Gas Desulfurization (FGD) sludge process was inspected on Wednesday, February 23, 2011. FGD sludge is disposed of in a pond located to the Northeast of Pond 6. This pond is incised and was not investigated further.

2.3 SIZE AND HAZARD CLASSIFICATION

The classification for size, based on the height of the dam is "Small" and based on the storage capacity criterion is "Small" in accordance with the USACE Recommended Guidelines for Safety Inspection of dams ER 1110-2-106 criteria summarized in Table 2.2a.

Table 2.2a: USACE ER 1110-2-106Size Classification						
	Impoundment					
Category	Storage (Ac-ft)	Height (ft)				
Small	50 and < 1,000	25 and < 40				
Intermediate	1,000 and < 50,000	40 and < 100				
Large	> 50,000	> 100				

The State of Texas maintains a Dam Safety program through the Texas Commission on Environmental Quality (TCEQ). The only non incised pond (Pond 6) falls in the small category and is not a part of the Texas Dam inventory. Also, the American Electric Power Services Corporation Oklaunion Power Station Wastewater Evaporation Pond 6 dam is not in the National Inventory of Dams and therefore does not have an established hazard classification. Dewberry conducted a qualitative hazard classification based on the 2004 Federal Guidelines for Dam Safety classification system (shown in Table 2.2b).

Table 2.2b: FEMA Federal Guidelines for Dam SafetyHazard Classification							
	Loss of Human Life	Economic, Environmental, Lifeline Losses					
Low	None Expected	Low and generally limited to Owner					
Significant	None Expected	Yes					
High	Probable. One or more expected	Yes (but not necessary for classification)					

Loss of human life is not probable in the event of a catastrophic failure of the embankment and a failure of the embankment is expected to have a low economic and environmental impact. Therefore, Dewberry evaluated the Wastewater Evaporation Pond 6 as "**low hazard potential**".

2.4 AMOUNT AND TYPE OF RESIDUALS CURRENTLY CONTAINED IN THE UNIT(S) AND MAXIMUM CAPACITY

The data reviewed by Dewberry did not include the volume of residuals stored in Wastewater Evaporation Pond 6 at the time of inspection.

Table 2.3: Maximum Capacity of Unit							
Wastewater Evaporation Pond 6							
Surface Area (acre) ¹	58.0						
Current Storage Capacity (cubic yards) ¹	Unknown *						
Current Storage Capacity (acre-feet)	Unknown *						
Total Storage Capacity (cubic yards) ¹	Unknown *						
Total Storage Capacity (acre-feet)	Unknown *						
Crest Elevation (feet)	Unknown *						
Normal Pond Level (feet)	Unknown *						

* Data was not provided by Utility

2.5 PRINCIPAL PROJECT STRUCTURES

2.5.1 Earth Embankment

The embankment is earthen filled with sandy clay with trace gravel. The approximate minimum designed crest width is 20 feet, however, since Pond 6 is in the process of being closed, the current crest widths exceed 150 feet with a minimum of 80 feet at its narrowest point. Approximate embankment height is 20 feet. The embankment is anchored into original ground at a minimum depth of five (5) feet. In-situ earthen material was used in the construction of the embankment according to the original specifications.

2.5.2 Outlet Structures

Water generated by Wastewater Evaporation Pond 6 is evaporated from the pond leaving the waste ash in place. The Pond does not have an outlet structure.

2.6 CRITICAL INFRASTRUCTURE WITHIN FIVE MILES DOWN GRADIENT

The American Electric Power Services Corporation Oklaunion Power Station is located southeast of Vernon Texas and south of Oklaunion Texas. Wastewater Evaporation Pond 6 is located on an unnamed tributary of Boggy Creek, Beaver Creek, Wichita River, and Red River. It cannot be determined if there is any critical infrastructure immediately downstream along the unnamed tributary, as this data was not provided by the Owner. Given the rural location of the plant, no critical infrastructure would be expected in the vicinity of the plant.

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3.0 SUMMARY OF RELEVANT REPORTS, PERMITS, AND INCIDENTS

Summary of Reports on the Safety of the Management Unit

Four quarterly inspection reports were provided by AEP:

- 2009 Dam & Dike Inspection Report, Wastewater Pond Complex, Oklaunion Power Plant, June 2009, Prepared by URS Corporation.
- Dam & Dike Inspection Report, Wastewater Pond Complex, Oklaunion Power Plant, Inspection Date: August 4, 2010, Prepared by William R. Smith, P.E.
- Inspection Report, Oklahoma Pond Complex, Inspected By: Pat Mackenzie and Steve Lewis / Oklaunion Power Plant, November 22, 2010.
- Inspection Report, Oklahoma Pond Complex, Inspected By: Pat Mackenzie and Steve Lewis / Oklaunion Power Plant, March 7, 2011.

3.1 SUMMARY OF LOCAL, STATE, AND FEDERAL ENVIRONMENTAL PERMITS

The State of Texas has a Dam Safety Program that is the responsibility of the Texas Commission on Environmental Quality (TCEQ); however, due to its small size, this dam is not permitted by the TCEQ.

Discharge from the impoundment is regulated by the Texas Commission on Environmental Quality (TCEQ). The TCEQ has issued a National Pollutant Discharge Elimination System Permit for the plant. Permit No. WQ0002574000 was originally issued September 13, 2004 and was re-issued on November 16, 2007.

3.2 SUMMARY OF SPILL/RELEASE INCIDENTS

Data reviewed by Dewberry did not indicate any spills, unpermitted releases, or other performance related problems with the dam over the last 10 years.

Oklaunion Power Station American Electric Power Vernon, Texas

4.0 SUMMARY OF HISTORY OF CONSTRUCTION AND OPERATION

4.1 SUMMARY OF CONSTRUCTION HISTORY

4.1.1 Original Construction

The Oklaunion Power Station Wastewater Evaporation Pond 6 was constructed in 1987. It is a side hill embankment.

4.1.2 Significant Changes/Modifications in Design since Original Construction

- The upstream slope has been filled and compacted with native soil dredged from adjacent process water evaporation ponds. It was reported that this material did not contain CCR.
- Dewberry personnel onsite were informed that Evaporation Pond No. 6 is in the process of being closed.

4.1.3 Significant Repairs/Rehabilitation since Original Construction

Notations have been made in the internal inspection reports over the past two years (2009 and 2010) of the following repairs / rehabilitation:

- Vegetation clearing
- Repair of eroded gullies on embankment slopes
- Removal of small trees from embankment slopes.

4.2 SUMMARY OF OPERATIONAL PROCEDURES

4.2.1 Original Operational Procedures

The Plant features 17 wastewater evaporation ponds. Eleven of these ponds receive cooling tower blowdown; no CCR is stored in these 11 ponds. Six of the ponds contain some byproducts of the coal combustion process. Five of these ponds are incised, and therefore have no embankments or dams. Only one evaporation pond is not incised - Pond 6. Earthen fill dikes retain the waste until it is sufficiently dry to be hauled away and landfilled or the waste is left in place and the ponds are closed as is the case with Evaporation Pond No. 6.

4.2.2 Significant Changes in Operational Procedures and Original Startup

No documents were provided to indicate any operational procedures have been changed.

4.2.3 Current Operational Procedures

Bottom Ash and boiler slag waste are wet conveyed to the Evaporation Pond Complex. Ash is deposited via sedimentation into the six CCR storage ponds. Ash sediment is currently stored in Pond No. 6, which will eventually be closed, and the ash reclaimed for beneficial use.

4.2.4 Other Notable Events since Original Startup

No additional information was provided to Dewberry of other notable events that have impacted the impoundment's operation.

5.0 FIELD OBSERVATIONS

5.1 PROJECT OVERVIEW AND SIGNIFICANT FINDINGS

Dewberry personnel Andrew Cueto, P.E. and Kyle Shepard, P.E. performed a site visit on Wednesday, February 23, 2011 in company with the participants.

The site visit began at 10:30 AM. The weather was cloudy and cool. Photographs were taken of conditions observed. Please refer to the Dam Inspection Checklist in Appendix C for additional site information. Selected photographs are included here for ease of visual reference. All pictures were taken by Dewberry personnel during the site visit.

The overall assessment of the dam was that it was in satisfactory condition and no significant findings were noted.

5.2 SOUTH EMBANKMENT

5.2.1 Crest

The crest of the South Embankment showed no signs of depressions, tension cracks, or other indications of settlement or shear failure, and appeared to be in satisfactory condition. Figure 5.2.1-1 shows the conditions of the crest of the South Embankment.



Figure 5.2.1-1. Figure showing the South Embankment crest and outside slope conditions.

5.2.2 Upstream / Inside Slope

The South Embankment's inside slope has been filled up to the crest elevation with native soil, effectively eliminating this slope on the east end of the embankment. Figure 5.2.2-1 shows these areas.



Figure 5.2.2-1. Figure showing the South Embankment lack of inside slope.

5.2.3 Downstream / Outside Slope and Toe

There were no observed scarps, sloughs, bulging, cracks, or depressions indicating slope instability or signs of erosion. The outside slope of the South Embankment was covered with sparse overgrown vegetation. The outside slope appears to be in good condition. Figure 5.2.3-1 shows the general condition of the outside slope of the South Embankment.



Figure 5.2.3-1 general good condition of the outside slope of the South Embankment.

5.2.4 Abutments and Groin Areas

The outside slope of the abutments and groin were uniformly graded and covered with sparse grass. Erosion or uncontrolled seepage was not observed along either groin. Figure 5.2.4-1 shows the general condition of the Abutments and Groin.



Figure 5.2.4-1. Southern Groin.

5.3 EAST EMBANKMENT

5.3.1 Crest

The crest of the East Embankment showed no signs of depressions, tension cracks, or other indications of settlement or shear failure, and appeared to be in satisfactory condition. Figure 5.3.1-1 shows the conditions of the crest of the East Embankment.



Figure 5.3.1-1. Figure showing the East Embankment crest and outside slope conditions.

5.3.2 Upstream / Inside Slope

The East Embankment's inside slope is non-existent with the waste in the pond being allowed to remain in place up to near the crest elevation of the embankment. The Pond appears to being allowed to fill up with solid ash waste working its way east to west. This condition can be viewed in Figure 5.3.2-1.



Figure 5.3.2-1. East Embankment, lack of inside slope.

5.3.3 Downstream / Outside Slope and Toe

There were no observed scarps, sloughs, bulging, cracks, or depressions indicating slope instability or signs of erosion. The outside slope of the East Embankment is the upstream slope of the Emergency Spillway. It is uniformly graded and covered with sparsely overgrown grass. Figure 5.3.3-1 shows the general condition of the East Embankment's crest, outside slope and groin.



Figure 5.3.3-1. Crest, outside slope and groin of East Embankment.

5.3.4 Abutments and Groin Areas

There were no observed scarps, sloughs, bulging, cracks, or depressions indicating slope instability or signs of erosion on the groins of the East Embankment. The top right-side of Figure 5.3.3-1 shows the general condition of the East Embankment's groin.

5.4 OUTLET STRUCTURES

5.4.1 Overflow Structure

The Overflow Structure is detailed in the original design drawings (See Appendix A – Doc 9). The structure is not on Wastewater Evaporation Pond No. 6, but is on Pond No. 7, immediately to the east of the East Embankment of Pond No. 6. Pond 6 is an evaporation pond with no outlet structures or spillways.

5.4.2 Outlet Conduit

No Outlet Conduit is present.

5.4.3 Emergency Spillway

See Overflow Structure, Paragraph 5.4.1 above.

5.4.4 Low Level Outlet

No Low Level Outlet is present.

6.0 HYDROLOGIC/HYDRAULIC SAFETY

6.1 SUPPORTING TECHNICAL DOCUMENTATION

6.1.1 Flood of Record

No documentation has been provided about the flood of record.

6.1.2 Inflow Design Flood

No documentation has been provided concerning the design flood.

6.1.3 Spillway Rating

No spillway hydraulic data was provided for review.

6.1.4 Downstream Flood Analysis

No downstream flood analysis data was provided for review.

6.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION

No documentation has been provided. Therefore, the supporting documentation reviewed by Dewberry is inadequate.

6.3 ASSESSMENT OF HYDROLOGIC/HYDRAULIC SAFETY

No documentation has been provided. Therefore, no assessment can be made.

7.0 STRUCTURAL STABILITY

7.1 SUPPORTING TECHNICAL DOCUMENTATION

7.1.1 Stability Analyses and Load Cases Analyzed

Original stability and load case design analyses were not provided by the utility.

7.1.2 Design Parameters and Dam Materials

Documentation provided to Dewberry for review was the Specifications for Phase I Construction (Site Preparation) Oklaunion Power Station (Unit No. 1) (See Appendix A – Doc. 4).

7.1.3 Uplift and/or Phreatic Surface Assumptions

No documentation of uplift calculations was provided to Dewberry for review. Based on the Geotechnical Borings included in the Specifications for Phase I Construction (Site Preparation) Oklaunion Power Station (Unit No. 1), (See Appendix A – Doc. 2), the initial phreatic surface was assumed to be at the elevation measured in the borings.

7.1.4 Factors of Safety and Base Stresses

No documentation was provided to Dewberry for review regarding safety factors or base stresses.

7.1.5 Liquefaction Potential

The documentation reviewed by Dewberry did not include an evaluation of liquefaction potential. Foundation soil conditions do not appear to be susceptible to liquefaction.

7.1.6 Critical Geological Conditions

There was no documentation provided to Dewberry that included an evaluation of Critical Geological Conditions.

7.2 ADEQUACY OF SUPPORTING TECHNICAL DOCUMENTATION

Structural stability documentation is inadequate.

Oklaunion Power Station American Electric Power Vernon, Texas



7.3 ASSESSMENT OF STRUCTURAL STABILITY

Overall, no assessment of the structural stability of the dam can be made because no documentation was provided. Based on observations made during the February 23, 2010 field visit by Dewberry and the fact that the operator indicated that Pond 6 was being closed, a structural stability report and analysis could be waived if a closure plan and schedule was provided.

The following observations were made during the onsite inspection:

- The crest appeared free of depressions and no significant vertical or horizontal alignment variations were observed,
- There were no major scarps, sloughs, or bulging along the embankments,
- Boils, sinks, or uncontrolled seepage were not observed along the slopes, groins, or toes of the embankments.



8.0 ADEQUACY OF MAINTENANCE AND METHODS OF OPERATION

8.1 OPERATING PROCEDURES

The facility is operated as a settling basin and storage of coal combustion ash residual deposits and bottom ash transport water.

8.2 MAINTENANCE OF THE DAM AND PROJECT FACILITIES

While no maintenance plan was supplied to Dewberry for review, based upon observations made during the February 23, 2010 site visit and discussions with Plant representatives, dam maintenance appears to be adequate.

8.3 ASSESSMENT OF MAINTENANCE AND METHODS OF OPERATIONS

8.3.1 Adequacy of Operating Procedures

No documented operational procedures were supplied to Dewberry for review. However, a verbal description of maintenance procedures and methods was presented at time of inspection.

8.3.2 Adequacy of Maintenance

No record of Maintenance was supplied to Dewberry for review. It was observed that the existing operating procedures adequately maintain the Management Unit. It was recommended that these procedures be documented and put into checklists.



9.0 ADEQUACY OF SURVEILLANCE AND MONITORING PROGRAM

9.1 SURVEILLANCE PROCEDURES

Quarterly Inspections

The plant surveillance procedures state, "Quarterly inspections shall be completed by Plant personnel and within 24 hours of unusual events, such as seismic activities or a "significant storm event". A significant storm event is defined as three inches or more of rainfall in 24 hours. Inspections should be documented in accordance to AEP Circular Letter CI-M-CL-010C."

9.2 INSTRUMENTATION MONITORING

No documentation was provided to Dewberry, nor was monitoring instrumentation seen during the site visit, to suggest that instrumentation monitoring was in place at Oklaunion Power Station.

9.3 ASSESSMENT OF SURVEILLANCE AND MONITORING PROGRAM

9.3.1 Adequacy of Inspection Program

Based on the data provided in the quarterly inspection reports reviewed by Dewberry, plus observations during the site visit, the inspection program is adequate.

9.3.2 Adequacy of Instrumentation Monitoring Program

An instrumentation monitoring program is not needed for the one nonincised pond, given the intention to close Pond 6 in the near future,



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	LOG OF BORING NO. 8-108 OKLAUNION POWER STATION UNIT NO. 1 WILBARGER COUNTY, TEXAS									
Ŧγ	TYPE BORING: Undisturbed Sample and Core LOCATION: See Fign of Sorings, Plate 3									
EPTH. FT.	SYMBOL	SAMPLES	SOIL DESCRIPTION	RQD %	CORE	// PASSING 0.200 SIEVE		PLASFIC	IOISTURE	SHEAR STRENGTH
		Ŷ	ELEVATION: 1201.2		2	Ξ	 	<u> </u>	20	0.5 10 15 3
- 5	R		Very stiff reddish-brown silty clay, w/occasional coarse gravel and organic material -w/numerous weathered calcareous nodules at 3.0' (CL)							
			Soff reddish-brown sandstone,							╄┽ <u>╄┽┼┼</u> ┝╄┥┝┿┥┝┥╴╶ <u>┨</u>
-10			-w/numerous soft bluish-gray sandstone laminations and <u>inclusions at 6,0</u> . Soft reddish-brown siltstone, thin flat bedding, w/numerous soft bluish-gray siltstone inclusions -soft bluish-gray siltstone seams, gently dipped bedding at 11.0' -clayey at 13.0' -high angle fractures -soft reddish-brown claystone		100 100 100	99 92	35 31	16 16	10 4	
20			-soft bluish-gray sandy claystone gravel -soft bluish-gray sandy claystone gravel -soft bluish-gray sandy claystone soom, w/fine gravel from 27.5- 28.5		00 00	86	29	14	8	
30-1	Ň	,	Very stiff raddish-brown silty	T					Ĥ	
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	TANA	; ; ; ;	ioft reddish-brown claystone, hick bedding, w/occasional oft bluish-gray silty claystone netusions							
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	LOG OF BORING NO. 8-108 (Cantd.) OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS									
DEPTH. FT.	SYMBOL	SAMPLES	SOIL DESCRIPTION	ROD %	CORE- RECOVERY %	% PASSING NO. 200 SIEVE	LIQUID	PLASTIC LIMIT	MOISTURE CONTENT %	SHEAR STRENGTH
		W 717 H W W	-w/numerous high angle fractures at 35.0'	79	80					
4			-w/numerous high angle fractures						-	
-45			-soft clayey shale seam from 43.0–44.0° -soft bluish–gray claystone seam from 44.8–45.15	70	100					
-50			-w/numerous high ongle fractures			:			بر - -	
			-exitempty stickensided				-			
			-soft bluish-gray claystone seam, w/fine gravel from 57.0-57.6"	93	100					
18			^o w/numerous slickensides, indurated							
3			-w/numerous high angle fractures	97	100		I			
	The second				 			-		
			(Contine	ved)						
ANTIONAL INIL DERVICES, INN. PLATE 25										





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	LOG OF BORING NO. 8-770 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
	۶Ę	BORING: Undisturbed Sample and Core L	<u>ộca</u>	tion	<u>: S</u> e	e Pl	an o	<u>f 30</u>	ring		<u>Plat</u>	e 3	3		
DEPTH. FT.	SOIL DESCRIPTION SOIL DESCRIPTION SOIL DESCRIPTION SOIL DESCRIPTION SOLUTION: 1213.0 LINE OF STRENGTH ALL LINE OF														
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+ 5 -		roots -reddish-brown, w/occasional coarsa sand and fine gravel, w/weathered colcareous nodules (C1) C													
	Soft reddish-brown silty day-														
	stone, w/numerous large bluish- gray silty claystone inclusions, w/some fine sand 10-555 Hard reddish-brown silty clay, 100														
		Iw/some fine sandHard reddish-brown silty clay, somewhat friableSoft reddish-brown silty clay- stone, crossbedded, w/accasional bluish-gray silty claystone inclusions-crossbedded, w/accasional bluish-gray silty claystone laminations and very soft clay- stone and increasing silt at 14.0' -some fine sand-w/numerous yellow inclusions, thick bedded, decreasing silt		100 100 100											╎┓╎╵╻┎╻╷╎╏┙┙┶┓╷╻╹╻╻╻╹╴
		-w/bluish-gray silty daystone seam at 30.1–30.4' w/coarse sond and fine gravel to 33.0'		100											
-35		Soft reddish-brown silty shale, w/accasional bluish-gray silty shale inclusions	5			- +-		- 11							
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	LOG OF BORING NO. 3-171 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
	TΥ	ÞE	BOR	ING:Ungisturbed Sample and Corel	.oca	TION	1: 5e	e ?!	an o	of Son	rings, Plate 3				
	DEPTH, FT.	SYMBOL	SAMPLES	SOL DESCRIPTION	KQD %	CORE RECOVERY %	% PASSING NO. 200 SIEVE	LIQUED	PLASTIC LIMIT	MOISTURE CONTENT %	SHEAR STRENGTH				
	5			Hard dark brown silty clay, w/weathered calcareous nodules and fine roots ~increasing calcareous nodules at 3.0' ~brown and fan, w/pockets of fine sand at 5.0'		92	_								
┸┺┶┷┺╼┶┊┊┊┊╵╏╏╷╷╻				-reddish-brown, w/occasional bluish-gray silty clay seams, <u>crossbedding at 7.0' (C1)</u> Soft reddish-brown silty clay- stone, w/soft to moderately hard clayey sandstone interbeds irom 7.0-12.0' -w/bluish-gray silty claystone		100									
				-crossbedded at 11.0' -w/some coarse sand and bluish- gray silty claystone inclusions at 15.0' -decreasing silt -thin flat badding		00									
-2				-w/bluish-grey silty claystone inclusions, crossbedded		100			1	┝╶╍╋╸ ┫╼╌┼ ┠╴┝╸╋					
				-w/bluish-gray silty claystone eam at 30.0-31.0'						┶┿╌┠╌┠╌┠╌┠ ┑╋╌┠╌┠╌┠╴┠ ┱╋╴┠╴╴┨╴					
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		CINAL	50K	Structs							PLATE 91				



	LOG OF BORING NO. 3-172 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
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			very stiff reddish-brown silly clay, w/accasional fine gravel												
	at 3.0' -hard, increasing coarse sand														
5	5 -hard, increasing coarse sand and fine gravel, very friable -gravel seam at 5.0-6.0' (C1)														
	-gravel seam at 5.0-6.0' (CL)														
-10-	Soft reddish-brown silty clay- 10-00 stone, thick bedded, w/occa- sional bluish-gray silty clay- stone inclusions														
<u>-15</u> -			w/moderately hard silistone soom at 14.0-14.2		100										
			-w/occasional bluish-gray silty claystone seams and inclusions at 15.0°		i			ļ	ļ						
-20-			-thíck bedding		100			ļ							
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-25			-w/numerous small bluish-gray silty claystone inclusions		100										
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30			-decreasing silt -severe water loss at 30.0'	-	100	ĺ			+ -						
		-	heavily argetured at 30.0-32.0'												
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***		<u>578</u> 4 69	SERVICES							PLATE 93.	ŧ				



	LOG OF BORING NO. 8-173 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
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	DEPTH. FT.	SYMBOL	SOIL DESCRIPTION	RQD %	CORE RECOVERY %	% PASSING NO. 200 SIEVE	LIQUID	PLASTIC	MOISTURE CONTENT %	SHEAR STRENGTH	L83./CU. FT.				
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			W/calcareous and magnesium nodulas and numerous fine raots (CL) Hard reddish-brown silty clay,		100										
) increasing weathered colcareous (<u>nadules</u> (<u>CL</u>) Soft reddish-brown clayey silt- stone, w/small bluish-gray clayey silttage inclusions	-	98										
	sillstone inclusions -slightly slickensided at 8.0' -w/increasing claystone faming tions, crossbedded at 11.0'														
	5 1		Soft reddish-brown silty clay- stone, w/bluish-gray silty clay- stone seam at 14.2-14.6'	ľ	95						للنلغينية				
			-w/accosional bluish-gray silty claystone inclusions and seams		96										
-2	2222227222	ACALACIES OF		ן ן ן	00										
-3		並將就在	-crossbecided, w/numerous large bluish-gray silty claystone		00				╊ ┻ ┥ ┝ ╋ ┝ ╋		<u> </u>				
-3:			-w/bluish-gray silty claystone seam at 33.0-33.5 ¹					, -			┍╌╸╸╸				
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I	LOG OF BORING NO. 8-179 OKLAUNION POWER STATION - UNIT NO. 1														
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77	P도 1	BORING: Undisturbed Sample and Care L		<u>1101</u>	l:Se	e Pla	an ol	f <u>So</u> r	ings, Plate 3						
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DE	~	ELEVATION: 1201.2	1 -	22	≈g	-	۹.	хÖ							
	N	Very stiff dark brown silty clay,	1						- 1.5000000000000000000000000000000000000						
	5	w/numerous fine roots (CL)					:								
	N	Hard reddish-brown silty clay,		!											
	N	w/calcareous and magnesium						ļ							
5	5 N 1000105														
	-w/occasional large calcareous 100														
년에	10-1 and magnesium nodules														
<u> </u> {															
	82	-w/numerous bluish-oray silby		85				h							
	N	_clay inclusions (CL)		1				ļ	┝┥┥┬╆╧┼╤╞╎╎┝┝╤┥╉╏╤┇╧┦╶╶┫						
		Saft reddish-brown silty clay- stone, crossbedded, w/occasional bluish-gray silty claystone inclusions		100											
25		-increasing silt	T	00											
30 120		-fhick bedded	1	00				┍╄╌╊┯╍┠╶╍┨							
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		silly cloystone inclusions	1	[1		H	┽╪╞╕┟╪╎╎╿┝┿┝╎┥╶┨						
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LOG OF BORING NO. 8-180 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
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	PE.	301	RING: Undisturbed Sample L			Se . u	e Plo	<u>an di</u>	Bori	SHEAR STRENGTH SHE				
EPTH. FT	SYMBOL	SAMPLES	SOIL DESCRIPTION	CORE ECOVERY	OWS PER F	% PASSING 0. 200 SIEV	LIMIT	PLASTIC	I OISTURE					
-ª	1.5	Ŷ	ELEVATION: 1198.4	e e		a.Z			20					
			Hard brown clayey silt, -w/numerous calcareous nodules and fine roots											
- 5 -			-w/large calcareous nodules of 3.0' (ML) Hard reddish-brown silty clay, w/numerous weathered calcaroous	80		, 90	Эб	10	13					
			nodules -w/large calcareous nodules, coarse sand, and medium to 7											
			coarse gravel at 6.0' -w/coarse gravel at 8.0' (CL) Soft reddish-brown silty clay-		ma									
-15			stone Inclusions		44									
-20-				100			104							
			-w/occasional high angle fractures -w/bluish-gray clay seam at	100			-							
			24.5-25.5 ³ Hard reddish-brown silty clay		D⁄4"									
-30-			-w/occasional bluish-gray silty											
35	翻		Soft reddish-brown silty clay- stone, thin flat bedding											
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LOG OF BORING NO. 8-182 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS															
TY	PE	30F	ואָק: Undisturbed Sample	LOCA	TIO	t∶Se	e 21a	an of	F Sori	ings, Plate 3					
EPTH. FT.	LI HIAN SOIL DESCRIPTION ELEVATION: 1203.7 LI HIAN SOIL DESCRIPTION LI														
		Ų	ELEVATION: 1203.7	ä	티골				Σō						
5			Hard brown silty clay, w/fine roots, accasional coarse sand and weathered calcareous nodules (CL) Hard reddish-brown silty clay, w/numerous weathered calcar- eous nodules -extremely friable, w/fine	, 100	,										
			grovel at 5.0" -w/some fine gravel and iron nadules at 7.0" -w/occasional bluish-gray siity clay inclusions at 10.0" -w/bluish-gray silty clay pockets (CL)	100	51/8 seat	99	40	20	12						
			Soft reddish-brown silty clay- stone, thin to medium bedding, w/numerous line roots	100		84	30	15	9						
-20-			-continuing bluish-gray pockets		51/5 seat										
-30-4			Very soft raddish-brawn clayey shale -w/bluish-gray silty clay inclusions	100											
			Soft reddish-brown silty clay- stone, thick bedded -slightly slickensided	100											
			(Contin	ued)											

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OF BORING NO. 3-182 (Contd.) LQG OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS i≽Ę, SHEAR STRENGTH ÷. SAMPLES DEPTH, FT MOISTURE CONTENT. PLASTIC LINUT % PASSIN NO. 200 SLE LINIT LINIT IN TONS/SQ FT SYMBOL ZU SU SU RECOVERN SOIL DESCRIPTION 1,5 Q.5 10 1 É -very soft clayey shale seam at 38.5-40.0 100 50 5.5 60 70 DEPTH TO WATER: Caved at 7.5 COMPLETION DEPTH: 40.0' DATE: 6/11/81 DATE: *5/5/*81 DI STE 114

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	LOG OF BORING NO. B-190 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS TYPE BORING: Undisturbed Sample LOCATION: See Plan of Borings, Plate 3																		
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7IPPETT & GEE, INC. CONSULTING ENGINEERS ABILEME, TEXAS

OK1-559.10 (OPS-1022)



<u>SPECIFICATIONS</u>

<u>F O R</u>

PHASE I CONSTRUCTION (SITE PREPARATION)

OKLAUNION POWER STATION

UNIT NO.1

DIVISION 1 - GENERAL

101. PROJECT DESCRIPTION

Central and South West Services, Inc., will construct a plant to be located in Wilbarger County, approximately four miles south of Oklaunion, Texas. The Jobsite may be reached by a fair-weather road. A vicinity map showing both road and railroad locations is given on page 1-2.

102. <u>PROJECT DATES</u>

The unit is scheduled for Commercial Operation on December 1, 1986.

103. EQUIPMENT/MATERIALS_SUBSTITUTION

Where specific brand names are used, they are a measure of quality only and equipment accepted as equal by Agent or Engineer may be substituted.

104. SITE CONDITIONS

Existing elevations are shown on the drawings. Solid lines are existing contours and dashed lines are proposed contours. Ambient temperature range is -5°F to 115°F. Seismic zone is '1".

US EPA ARCHIVE DOCUMENT

Drawing Number	Revision	Title
C-102-073	0	Culvert Details
C-102-074	0	Collection Pond Overflow Jetails
C-102-075	0	Collection Fond Overflow Details
C-103-023	0	Barbed Wire Fence Details
C-103-024	Û	Project Sign Elevation Sects. & Dets.
C-103-025	0	Fence Details
C-103-026	Û	Fence Details
A-110-001	۵	Prefabricated Guard Building

202. GENERAL SCOPE DEFINITION (CONTRACTOR)

The following is intended to provide a general definition of the scope of the Work under this Contract and shall not be construct as an itemized listing. Contractor shall be responsible for construction of complete facilities conforming in all respects to the details and requirements of this Specification, Drawings and other Contract Documents.

- 202.01 Clearing and grubbing of ground surfaces within the construction areas as delineated on the Drawings.
 - .02 Excavation, backfill, compaction and grading of generation plant area, chimney area, cooling tower area, switchyard area, perimeter fence area, pond areas, coal storage and handling areas.
 - .03 Construction of one (1) upper collection pond, one (1) lower collection pond, one (1) make up water supply pond, five (5) storage ponds and ten (10) evaporation ponds. Pond construction includes all excavation, backfills, embankments, slope protection, miscellaneous spillway and outfall structures, and any required linings.
 - .04 Construction of Jobsite railroad subgrade, plant access roads, roads and construction parking lot including all excavation, fills, subgrade preparation, lime stabilization, fly ash stabilization, surfacing, embankments and culverts.
 - .05 Final grading of all ground surface areas disturbed by the construction to provide uniform surfacing and effective drainage of ground areas.
 - .06 Construction of diversion ditches and required construction containment areas.
 - .07 Supply and erect one (1) prefabricated guard building complete with concrete slab foundations

OIVISION 4 - TECHNICAL REQUIREMENTS

401. JO8SITE WORK

401.01 Applicable Codes and Standards:

All references to the following publications are to be latest issue of each, together with the latest additions and/or amendments therato, as of the effective date of Contract. References to the sponsoring agencies will be made in accordance with the abbreviations indicated.

<u>ASIM</u> - American Society for Testing and Materials Standard Specifications

ACI - American Concrete Institute

AASHIQ - American Association of State Kighway and Transportation Officials Standard Specifications

THD - Texas State Department of Highways and Public Transportation

TEX - Standard Specifications Texas State Department of Highways and Public Transportation, Test Method

.02 Lines and Grades:

Contractor shall employ a competent, licensed land surveyor to determine lines and elevations.

Contractor shall lay out lines and grades from existing property lines and bench marks on the Jobsite and be fully responsible for correctness of such lines and grades and for proper execution of the Work to such lines and grades. Contractor shall correct at its own expense all errors in the Work arising from its inaccuracy.

Agent reserves right to verify correctness of lines and grades during progress of the Work. Such verification by Agent will not relieve Contractor of its responsibility.

Contractor shall preserve and maintain all bench marks and reference points established by Agent. Should Contractor, during execution of the Work, destroy or remove any bench marks and/or reference points established by Agent, the cost to Agent of reestablishing these bench marks and/or reference points will be charged to Contractor.

401.03 Surface Preparation:

Prior to performing excavation or fill work, the ground surfaces within the construction area shall be cleared, grubbed and the top-soil removed.

OK1-559.10 (OPS-1022)

TECHNICAL REQUIREMENTS

401.03 a.

> Clearing is defined as removal and disposal of all trees, down timber, snags, brush, hedges, bushes and all other vegetation or organic materials, and also all rubbish, debris or other foreign or objectionable materials above ground surface, except removal of sod and topsoil.

b. Grubbing:

Clearing:

Grubbing is defined as removal and disposal of all stumps, large roots, buried logs and all other objectionable material from below ground surface. Explosives may be used only if specifically approved and their use shall conform to all applicable laws and safety regulations.

c. Disposal:

All materials from clearing and grubbing operations shall be promptly removed from the construction area and disposed of by and at the expense of Contractor. Accumulation of such materials on premises is not permitted. If Contractor desires to burn materials, the obtaining of a burning permit from the appropriate regulatory authorities shall be Contractor's responsibility.

d. Removal of Topsoil:

Topsoil in areas to be excavated or filled shall be removed, stockpiled and saved at various locations on the Jobsite near where it is to be reused. The depth of topsoil to be excavated or stripped is estimated to be six to twelve incres in depth. Stockpile locations shall be approved by Agent. Topsoil shall be carefully stripped, placed in stockpiles and kept clean and free of all foreign material. Topsoil shall be utilized where indicated on the Drawings and required by this Specification (reference Section 404.02, c.).

401.04 Excavation:

Excavation is defined to include all incidental clearing, all disposal of excavated materials, all protection, sheeting, shoring, bracing, all dewatering, and preparation of bearing areas as required to properly install and complete the Work, regardless of portions of Work for which required, and regardless of nature of materials, encountered in excavating.

0K1-559.10 (0PS-1022)

401 04 a.

Protection and Support:

Banks at excavations shall be protected and supported, where necessary or where requested, so that the banks and bottoms will be maintained and adjacent structures or other construction will be protected from damage caused by an earth movement.

- b Bearing Areas:
- b.01 Bearing areas for all foundations shall be inspected and approved by Agent before any concrete is placed. If bearing areas are not suitable, Contractor may be requested to carry the excavations desper to more suitable bearing material.
 - .02 All foundations shall be placed on undisturbed soil unless otherwise indicated or approved.
- 401.05 Structural Backfill:

Structural backfill includes general backfilling around all Work excavated by Contractor, and also all other backfill indicated on Drawings as by Contractor. All backfill or ordinary fill shall be classified as structural backfill as defined herein unless indicated otherwise on the Drawings.

a. Materiai:

Unless indicated otherwise on the Drawings, all backfill material shall be approved materials previously excavated at the Jobsite or materials obtained from approved borrow pits and shall be free of sod and topsoil or other deleterious or foreign matter. This material shall be approved by Agent.

b. Compaction:

Sackfill shall be built up to the grade elevations indicated on the Orawings or as directed, with suitable moisture control and compaction throughout placing. All backfill soils and embankments, unless indicated otherwise on the Drawings, shall be compacted equal to or greater than 95 percent of the maximum dry unit weight as defined by the Standard Compaction Test (ASTM D-698) for plastic clays and the TRD Method (TEX-113E) for sandy clays, clayey sands, silty sands and sands. The moisture content may vary from two percent dry of optimum to four percent wet of the optimum when compacted. The thickness of backfill lifts shall not exceed 9".

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401.05 c.

Inspection and Testing:

Contractor shall perform all backfill testing necessary to ensure the Work meets the specified requirements. Agent will conduct a quality control test monitoring program in addition to Contractor's basic testing program. Failure of backfill to meet the specified densities will be cause for rejection, and Contractor will be required to remove and replace all backfill not meeting these requirements at no additional cost to Agent.

d. Backfill Sand:

Backfill sand shall be a free-flowing clean sand of fairly uniform gradation. Clay and soil content shall be absent in any quantities that in the opinion of Agent makes use of the material undesirable. The sand shall be vibratory compacted greater than 95 percent of the maximum dry unit weight obtained using THD Compaction Test Method (TEX-1135).

e. Embankments:

Embankments are defined as any fill section. This definition includes the pond embankments, railroad embankments, road embankments or any area requiring a build-up of soil.

Prior to construction of any embankment, the foundation area shall be stripped to remove the topsoil, roots and weak surface soils. The stripping dapth will be six to twelve inches, except in areas of uneven topography or in existing stock ponds and gullies. For these areas it will be necessary to strip to a deeper depth to remove the excessively wet and weak soils. All stumps and large roots shall be removed to a depth of two feet. All stripped areas shall be inspected by the on site testing laboratory and Agent's remove weak or otherwise objectionable materials that would adversely affect the intended integrity of the embankment.

After completion of stripping, the exposed soils shall be scarified to a depth of nine inches; the moisture shall be adjusted to two to four percentage points above optimum, and the soils recompacted. The optimum moisture value, compaction density and construction of embankment shall be as defined in the Structural Backfill Section 401.05 of this Specification.

f. Cut and Fill Slopes:

All slopes except pond slopes shall have a maximum inclination of 2.5 horizontal to 1.0 vertical or as indicated on the Drawings.

OK1-559.10 (OPS-1022)

401.05 f.

All exposed slope surfaces shall be cultivated, seeded and mulched as defined in the Seeding Section 404.02.

401.06 Grading:

Consists of rough grading and finish grading as follows:

a. Rough Grading:

Cut, fill, spread and level during course of Work to elevations indicated.

Finish Grading:

After all excavation, backfill, compaction and rough grading has been completed, all ground surface areas disturbed by Contractor shall be finish graded. The grading shall be finished to the contours and elevations indicated on the Grawings and shall provide a smooth uniform surfacing free of debris, foreign matter, objectionable stones, clods, lumps, pockets or high spots and properly drained to provide effective drainage of the ground areas. Finish elevations shall be within two inches (plus or minus) of those elevations shown on the Drawings in all areas except the power block (turbine generator area, boiler, scrubber, and chimney) and the switchyard area. Finish elevation for the power block and switchyard area shall be within one inct (plus or minus).

- C. An excess of backfill materials is anticipated. This excess material shall be utilized in a manner acceptable to Agent. The excess backfill shall be placed at locations approved by Agent. Oisposal areas that are proposed by Contractor and approved by Agent shall be shaped, compacted and seeded as directed by Agent. Disposal areas that affect pond capacities or plant operations will not be approved. Seeding and compaction of backfill shall be included in the total Contract Price. Unit prices are not applicable to the disposal of excess backfill.
- 401.07 Pond Construction:

Pond construction shall conform to the shapes, locations and dimensions as shown on the Drawings, to this Specification and the items described as follows:

a. Criteria:

Construction of all ponds shall conform to current criteria established by the Texas Department of Water Resources (TDWR) relative to waste ponds.

401.37 Þ.

Slopes:

Interior and exterior slopes shall be no steeper than 3.0 horizontal to 1.0 vertical.

c, Pond Sottoms:

Elevation of pond bottoms for the evaporation ponds, makeup supply pond and the collection ponds are indicated by existing contours or excavated to elevations shown on the Drawings - whichever is lowest. The finish grades or elevations of pond bottoms for the five (5) storage ponds shown on the Drawings reflect finish grades

Pond bottoms in cut areas shall be scarified to a depth of nine inches. The moisture content shall be adjusted to between two to four percentage points above the optimum value. Then the scarified subgrade shall be compacted. The optimum moisture value and compaction density shall be as defined in the Structural Backfill Section 401.05. Within 48 hours of compacting the pond bottom, a i2-inch layer of semi-compacted clay shall be spread over the entire bottom of each pond bottom in cut areas only to preserve the moisture content of the subgrade. Semi-compacted clay is defined as being greater than 90 percent of the maximum dry unit weight as defined by THD test method TEX-113E. The top of the semi-compacted clay shall be set at finish grade; therefore, for cut areas, Contractor shall overexcavate pond bottoms one foot to properly achieve finish grade.

d. Pond Embankments:

Pond embankments shall be constructed as defined in the Embankments Section 401.05.

·--.. B.

time Stabilization of Slopes:

The slopes shall be lime stabilized as indicated on the Orawings. The percent lime and method of placement of lime shall be as defined in the Lime Stabilization Section 404.03. The lime stabilized materials shall be placed in lifts such that the thickness of the lime stabilized materials is two feet measured perpendicular to the slope.

f. Seeding:

The pond slopes shall be seeded as indicated on the Drawings and as required by this Specification. The seeding shall be as defined in the Seeding Section 404.02

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401.07

g.

Pervious Soils:

All pond bottoms and side slopes must have a permeability coefficient less than 1 x 10⁻⁷ cm/sec. Out of numerous permeability tests conducted during the initial soils investigation only one -7 site demonstrated a permeability coefficient greater than 1 x 10⁻⁷ cm/sec and was considered pervious.

in the event that zones of pervious scils are disclosed in the pond bottoms or side slopes, these pervious zones must be removed by overexcavation to a depth of two feet and replaced with impervious materials. The placement and compaction of these impervious materials shall be as defined in Structural Backfill Section 401.05. The total contract price shall provide for excavating 50,000 cubic yards of pervious materials and replacing with 50,000 cubic yards of impervious materials. Unit prices shall be utilized for quantities above or below the 50,000 cubic yards that is included in the total Contract Price. The unit prices per cubic yard means to excavate one cubic yard of pervious materials and replace with one cubic yard of impervious materials.

401.08 Erosion and Sediment Control Requirements:

Contractor shall implement erosion and sediment control procedures as defined herein and shown on the Drawings.

a. Limitations:

The rainfall runoff from construction activities has the following limitations: 1) Total suspended solids - daily maximum of 50 milligrams per liter and 2) pH - between 6.0 and 9.0.

Contractor is required to conduct earthwork operations in such a manner than these limitations are met. To meet these limitations, Contractor shall comply with the following procedures:

Procedures:

Contractor shall conduct earthwork operations in the following sequence:

b.01 First - Contractor shall construct the three diversion ditches. Currently, approximately 150C acres of surface area off the Jobsite drain through the Jobsite. Also, there is considerable acreage in the Unit No. 2 evaporation pond area that will be unaffected by Contractor. These areas must be allowed to flow through the plant site unimpeded and unaltered.

401.08

US EPA ARCHIVE DOCUMENT

Second - Secondly and concurrently to the first procedure. Contractor shall construct improvement to the diversion ditches to ь.02 force certain area runoff into the diversion ditches. Accordingly, it is necessary to construct some of the access road with drainage structures to allow drainage of the Unit No. 2 evaporation pond area into diversion ditch number 2. It is also necessary to construct a portion of the railroad berm in Sections 42, 44, 45, 56, 57 and 58 to force runoff water from the north of the Jobsite to flow through diversion ditch number 1. Similarly, some of the west berms of evaporation ponds numbers 8, 9 and 10 must be constructed to force runoff water from the west of the Jobsite to flow into diversion ditch number 3. Contractor shall make all necessary improvements to the diversion ditches to allow areas that are unaffected by construction to flow through the plant site unimpeded and unaltered.

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- .03 <u>Third</u> Contractor shall construct the makeup water supply pond, the upper and lower collection ponds, evaporation pone number 4 and other ponds sufficiently to allow their use as sediment ponds. Contractor shall add temporary construction berms and diversion ditches as required such that runoff from all earthwork activities would drain into one of these ponds. Contractor may also install permanent drainage features as necessary to achieve these drainage/collection requirements.
- .04 <u>Fourth</u> Contrictor can then execute any required earthwork activity - if the rainfall runoff is directed into one of the sediment ponds.
- c. Pumpiny Requirements:

As water and sediment builds up in a particular sediment pond and the water has been clarified (contains less than 50 milligrams of suspended solids per liter) and as directed by Agent, it shall be pumped out by Contractor. The water shall be pumped into a diversion ditch or an existing stream that leaves the Jobsite.

Additionally, Contractor shall cooperate with Agent by pumping water between sediment ponds if required and as directed by Agent.

d. Cleanout of Sediment Ponds:

When all Work has been completed. Contractor shall pump out sediment ponds after they meet the required limitations and remove sediment if necessary to establish the required pond bottom elevations. Spoil areas for sediment shall be determined by Agent.

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404.02 d.

Sideoats Grama (El Reno) and 5 pounds per acre of Caucasion Bluestem.

e. Fertilization:

Fertilizer rates, seed purity, and seed germination rates will be in accordance with current THO Specifications, Item 164 and Item 166. The fertilizer shall have an analysis of 16-20-0 and shall be applied uniformly at the average rate of 300 pounds per acre.

404.03 Lime Stabilization:

Where indicated on the drawings line stabilized materials shall be provided in accordance with the following procedure:

Excavate the existing clay soils to the depth indicated on the drawing and stockpile the excavated material.

Scarify the top six inches of the subgrade to receive lime stabilized materials, add moisture as required and compact to a density not less than 95 percent of the maximum dry unit weight as defined by the THD Compaction test. (TEX-113E) and ASIM D-698. The moisture content should be between optimum and four percent above optimum at the time of compaction.

Treat the excavated clays with the required percentage of lime, (approximately 4% by dry weight) add water as necessary, and recompact at a moisture content between optimum and four percent above optimum, as defined by the THD compaction test, TEX-113E and ASTM D-698. The fills should be placed in loose lifts not exceeding nine (9) inches in thickness and compacted to a minimum density of 95 percent of the maximum as determined above. Failure to have adequate moisture content during mixing and compaction shall be cause for rejection of the Work.

Lime treatment (pulverization, incorporation of lime, repulverization, curing, etc.,) shall be accomplished in accordance with the applicable provisions of Item 260 of the THD Standard Specifications for Construction of Righways, Streets, and Bridges, 1972 Edition.

Seal the treated area of road beds and foundation areas with an asphalt membrane using .40 gallons of asphalt per square yard of surface - if the line stabilized materials are to be left exposed for more than seven (7) days. Sealing with asphalt is rot required for the lime stabilization areas on pond slopes.

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DIVISION 6 - GEOTECHNICAL INFORMATION

601. GENERAL

Agent has previously conducted a subsurface soil investigation. This section provides Contractor with information pertaining to the results of that investigation.

602. POSSIBLE HARD MATERIALS

The subgrade soils are hard. These soils may prove difficult to excavate and difficult to use as backfill materials. From the soil borings operations, it is anticipated that ordinary earth moving equipment can excavate the materials without blasting, even though blasting may prove advantageous to Contractor. It also appears that these hard soils should break down rapidly when exposed to sunlight and could then be broken apart and used as backfill material.

It is incumbent upon Contractor to satisfy himself as to his ownability to excavate and compact these hard soils. Arrangements can be made by Contractors to bring equipment on site to experiment with these soils prior to submitting proposals.

603. GEOTECHNICAL DATA

- 603.01 At the end of this section there are copies of the borings made at the Jobsite. The location of the borings are shown on the Drawings. This information is furnished for Contractor's convenience. Contractor will be permitted to make his own soil investigations, but same shall be made at no cost to Agent.
 - .02 From these logs, Contractor should determine the materials which Contractor will be excavating and compacting, apparent water table levels, possible sources of usable backfill materials and other pertinent information associated with his different construction techniques.

603.03 The following information is included hereinafter:

- Rock and soil classification.
- Maps of boring locations.
- c. Various soil profiles.
- d. Bering numbers.

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GEOTECHNICAL INFORMATION

OK1-559.10 (OPS-1022)

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603.04

The following is a listing of the boring logs which follow:

0 1CC	0.0000
0-133	8-203P
8-156	8-204
B-157	8-205
8-158	8-206
6~159	8-207
8-160	8-208
B-16*	9.200
9 1520	D 210
0 162	0→21U
8~103	, 8-211
8-164	8-212
8-165	8-213
8-156	8-214
8-167	B-215
8-168	8-215
8-169P	8-217
8-170	B-218
8-171	8-219
8-172	8-220
8-173	8-221
Q-174	0-221
0-1/4	D-222
0+1/3	B-223
0+1/6	B-224
B-177P	8-225P
8-178	8-226
B-179	B-227
B-180	B-228
B-181	B-229
B-182	8-230
9-183	8-231
8-185	8-232
8-186	8_233
8-190	B_234
8-102	0 225
0 100	0-235
B-193	5-230
B-194P	3-23/
3-195	B-239
B~196	B-240
B-198	B-241
B-199	B-242
8-200	8-243
8-201	8-244
B-202	•
	8-155 8-156 8-157 8-158 8-159 8-160 8-161 8-162P 8-163 8-164 8-165 8-166 8-167 8-168 8-169P 8-170 8-170 8-170 8-172 8-173 8-174 8-172 8-173 8-174 8-175 3-176 8-177P 8-178 8-179 8-185 8-185 8-185 8-185 8-185 8-190 8-194P 3-195 8-199 8-199 8-200 8-201 8-202

	LOG OF BORING NO. 8-108 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS												
TY	ÞE	805	ung:Undisturbed Sample and Core L	400	ייי	4: S	- Pf		f 80-	iont Plate 3			
DEPTH. FT.	SYMBOL	SAMPLES	SOIL DESCRIPTION	RQD %	CORE	% PASSING 0.200 SIEVE		PLASTIC LIMIT	JOISTURE	SHEAR STRENGTH			
E	N	¥	ELEVATION: 1201.2			Ī	 		<u>≥</u> 0.				
-5	8		clay, w/occasional coarse gravel and organic material "w/numerous weathered calcareous nodules at 3.0' (CL)					 					
			fine grained, thin bedding			İi			[<u><u></u></u>			
			-w/numerous soft bluish-gray sandstone laminations and <u>Inclusions at 6,0</u> Soft reddish-brown siltstone, thin flat bedding, w/numerous soft bluish-gray siltstone inclusions -soft bluish-gray siltstone seams, gantly dipped bedding at 11.0' -clayey at 13.0' -high angle fractures -soft reddish-brown claystone seam, thick bedding from 18.0- 20.7' -soft bluish-gray sandy claystone seam from 20.7-21.0', coarse gravel		100 100 100 100 100	99 92 86	35 31 29	16 16	10				
	Ň	1	(8.5'			-+	╌╀╸	+	<u> </u> 				
	ANNAY A		ioft reddish-brown claystone, hick bedding, w/occasional oft bluish-gray silty claystone netusions						╋╼┾╌╋╼╃╼				
			·····		ŀ			1					
		d. t	(Continue	sd)				_					
NAN YAN MUC	PLATE 24												

- <u>1</u>. (

			LOG OF BO OKLAUNION PO WILBARO	DRI WES GER (N G R STA COL	NÖ ATIC INT	. 8-)n - r, ti	-108 - UN EXAS	(Car IT N S	ntd.) IO. 1		
DEPTH. FT.	SYMBOL	SAMPLES	SOIL DESCRIPTION	ROD %	CORE- RECOVERY %	% PASSING NO. 200 SIEVE	LIQUID	PLASTIC LIMIT	MOISTURE CONTENT %	SHEAR STRENGTH		
		W 717 H W W	-w/numerous high angle fractures at 35.0'	79	80							
4			-w/numerous high angle fractures						-			
-45			-soft clayey shale seam from 43.0–44.0° -soft bluish–gray claystone seam from 44.8–45.15	70	100							
-50			-w/numerous high ongle fractures			:			بر - -			
			-exitempty stickensided				-					
			-soft bluish-gray claystone seam, w/fine gravel from 57.0-57.6"	93	100							
18			^o w/numerous slickensides, indurated									
3			-w/numerous high angle fractures	97	100		I					
	The second				 			-				
	(Continued)											
-4110-4	ATIONAL JULI, MERVICES, 1993; PLATE 25											







	LOG OF BORING NO. 8-170 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS														
	¢۶	BORING: Undisturbed Sample and Core L	<u>ộca</u>	tion	<u>: S</u> e	e Pl	an o	<u>f 30</u>	ring		<u>Plat</u>	e 3	3		
DEPTH. FT.	SYMBOL		RQD %	CORE CORE	% PASSING VO. 200 SIEVE		PLASTIC	MOISTURE CONTENT, %	\$i 	HEA IN T	R ONS	STA: 5750	ENGT ST	Ξ ΝΙ <u>Υ ΩΥ ΨΤ</u>	BS./CU. FT.
		Head because of the slow of fine	1 •••••	<u> </u>					t I	<u>as</u> 1111	i i i	0 1111		<u> ⊇</u> .	-
+ 5 -		roots -reddish-brown, w/occasional coarsa sand and fine gravel, w/weathered colcareous nodules (C1) C													
	Æ	Soft raddish-brown silty day-		100							<u>++</u> +	┼∔┼			1
	N	stone, w/numerous large bluish- gray silty claystone inclusions,		100				 [6t I		
		Iw/some fine sandHard reddish-brown silty clay, somewhat friableSoft reddish-brown silty clay- stone, crossbedded, w/accasional bluish-gray silty claystone inclusions-crossbedded, w/accasional bluish-gray silty claystone laminations and very soft clay- stone and increasing silt at 14.0' -some fine sand-w/numerous yellow inclusions, thick bedded, decreasing silt		100 100 100											╎┓╎╵╻┎╻╷╎╏┙┙┶┓╷╻╹╻╻╻╹╴
		-w/bluish-gray silty daystone seam at 30.1–30.4' w/coarse sond and fine gravel to 33.0'		100											
-35		Soft reddish-brown silty shale, w/accasional bluish-gray silty shale inclusions	5			- +-		- 11							
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<u> </u>	<u> </u>				[1		_1	11	111				· · · ·	ſ
		(Conti	nued	6)						_					
447.5% ت	110	. SDR. 55700C29 Ni Duimeleng								F	N.A.	E 8	9		-

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		LOG OF BORING NO. 3-171 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS												
	ΥT	ÞE	BOF	UNG:Ungisturbed Sample and CoreL	OCA	TION	1: 54	20 21	- aa o	of Son	rings. Plate 3			
	DEPTH, FT.	SYMBOL	SAMPLES	SOIL DESCRIPTION ELEVATION: 1910_3	RQD %	CORE RECOVERY %	% PASSING NO. 200 SIEVE	LIQUE	PLASTIC	MOISTURE CONTENT %	SHEAR STRENGTH	es./cu. FT.		
		the fille		Hard dark brown silty clay, w/weathered calcareous nodules and fine roots ~increasing calcareous nodules at 3.0" ~brown and fan, w/pockets of										
<u>┥┙┛┙┙┙┙┙┙┙┊┊┊┊╵╹╹╹╹</u>				-reddish-brown, w/occasional bluish-gray silty clay seams, <u>arcssbedding at 7.0' (CL)</u> Soft reddish-brown silty clay- stone, w/soft to moderately hard clayey sandstone interbeds irom 7.0-12.0'		100						. . [
				-w/bluish-gray silly claystone seams, w/accarse sand at 3,9-9,1" -cressbedded at 11.0" -w/some coarse sand and bluish- gray silly claystone inclusions at 15.0" -decreasing silt -thin flat bacding		100				┍╸╸ ╶╶╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴		<u></u>		
-2				-w/bluish-gray silty claystone inclusions, crossbedded		100				┝╶┤ ┍ ┛╺╴┥ ┇╴┤╺╸ ┲				
		ALL DIVERSION		-w/bluish-gray silty claystone icam at 30.0-31.0'				-		┷╼╼╊╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋╸╋				
_	7	(Continued)												
	(Continued)													



	LOG OF BORING NO. 3-172 OKLAUNION POWER STATION - UNITINO, 1 WILBARGER COUNTY, JEXAS											
	/ 25	000			-101	· · ·		·	- 6 P.	antara Miria o		
1		5 5 1 1 1	Undisturbed Sample and Corel		10× 10×	IEVE C	ee r		T %	SHEAR STRENGTH	i II	
DEPTH.	SYMB	SAMPI	SOIL DESCRIPTION	RQD	COR	% PASS 0.200 9	LIQUI	PLAST	A OISTU		85./CU	
<u> </u>		Y	ELEVATION: 1207.4	1		` 2				<u> 05 10 15 ≦</u> 	귀	
			very stiff reddish-brown silly clay, w/accasional fine gravel									
			cincreasing silt and gravel at 3.0'									
5	N		-hard, increasing coarse sand and fine gravel, very friable		100							
	2		-gravel seam of 5.0-6.0" (CL)								-	
-10-			stone, thick bedded, w/occa- sional bluish-gray silty clay-		100		1				لعلاة	
			stone inclusions -increasing silt and crossbedding		i				ļ	╪╪╪╪╷╎┍┶╌┝┯╼╌┝┝╎┍ ┙┙┙┙┙┙		
<u>-15</u> -			w/moderately hard silistone soom at 14.0-14.2		100							
			-w/occasional bluish-gray silty claystone seams and inclusions at 15.0°		i			ļ	ļ			
-20-			-thíck bedding		100			ļ				
							Ì			┝┥┙┛┚╴╴╴ ╕╴╴╴╴╴ ┇╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴		
-25			-w/numerous small bluish-gray silty claystone inclusions		100							
							ļ	ļ				
30			-decreasing silt -severe water loss at 30.0'	-	100	ĺ			+ -			
		-	heavily argetured at 30.0-32.0'									
35	(175) 					_		-				
											1	
			(Conti	nued)	}							
***	COMPATIONAL STAL STANDERS PLATE 93											



		LOG OF BORING NO. 8-173 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS												
	<u>1</u> Y	<u> 95 </u>	BORING: Undisturbed Sample and Core L	OCA	TION	<u>;</u> Se	e P!	an a	f Sar	ings, Plate 3				
	DEPTH. FT.	SYMBOL	SOIL DESCRIPTION	RQD %	CORE RECOVERY %	% PASSING NO. 200 SIEVE	LIQUID	PLASTIC	MOISTURE CONTENT %	SHEAR STRENGTH	L83./CU. FT.			
Ē		X	Hard derk brown silfy clay,											
			W/calcareous and magnesium nodulas and numerous fine raots (CL) Hard reddish-brown silty clay,		100									
) increasing weathered colcareous (<u>nadules</u> (<u>CL</u>) Soft reddish-brown clayey silt- stone, w/small bluish-gray clayey silttage inclusions	-	98									
			-slightly slickensided at 8.0' -w/increasing claystone lamina- tions, crassbedded at 11.0'		100									
	5 1		Soft reddish-brown silty clay- stone, w/bluish-gray silty clay- stone seam at 14.2-14.6'	ľ	95						للنلغينية			
			-w/accosional bluish-gray silty claystone inclusions and seams		96									
-2	2222227222	ACALACIES OF		ן ן ן	00									
-3		並將就在	-crossbecided, w/numerous large bluish-gray silty claystone		00				╊ ┻ ┥ ┝ ╋ ┝ ╋		<u> </u>			
-3:			-w/bluish-gray silty claystone seam at 33.0-33.5 ¹					, -			┍╌╸╸╸			
-					1					╈╫┲┥┲┚╎╏╎╋┿┽╎╍┽╎╋╡ ╺╢┧┨╎┇╷┯┱┥╽╽╎╄╦╤╻┲┨				
			(Continu	(beu						······································				
N a rs	5/54T	10я.ц.	504. 328mcE3				_			PLATE 95	1			







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PLATE 103







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LOG OF BORING NO. 6-179 OKLAUNION POWER STATION - UNIT NO. 1										
		WILBARGE	RCO	UNI	Υ,	TEX4	\$		• • • • • • • • • • • • • • • • • • •	
TY	- - -	BORING: Undisturbed Sample and Care	LOÇA	10	l: Se	e Pi	an ol	f Sor	ings, Plate 3	
EPTH, FT.	SYMBOL	SOL DESCRIPTION	RQD %	COVERY %	PASSINS 200 SIEVE	LIQUID	PLASTIC	DISTURE	SHEAR STRENGTH	
ā		ELEVATION: 1201.2	_ <u> </u>	8	°×5	:		žS	0.5 1.0 1.5 5	
		Very stiff dark brown silty clay,	ļ							
	N	Hand rod the brane site star	/						┿┥┇┇╏╵┊╎╴╎╄┥┿┱╦╫╾┪┯┫ ╶╴	
	XX	Weatersons and momentum								
	N	nodules						Ì		
-10-		-w/occasional large calcareous and magnesium nodules		100 100						
E					Í				╏╏┇╻┍┨┇┥┨╺╋╋╍╗┊╎╎╎┥┥ ╴╴╸ ╴╴╴╴╴	
-15-	X	-w/numerous bluish-groy silty clay inclusions (CL)		85	ļ			ļ		
20		Saft reddish-brown silty clay- stone, crossbedded, w/oacasional bluish-gray silty claystane inclusions		100						
		-increasing silt		00						
30-12		-fhick bedded	1	00						
出 (1)	圀							Ţ		
<u> </u>	圀	-this flat bedding	ļ			1		ļļ		
	訠	silly claystone inclusions	i	[į		H	┽╪╞╕┟┾╎╎╿┝┿┝┊┥╶╶┨	
35-22			-+	-+-	+	-+-	+	╇		
					ļ	ł		Hi	┼╁╏┼╎┼┼╞╎┟┿┥╶╶┨	
<u> </u>	╧╦┫ <u>╶╶┊╏</u> ╶╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴╴									
		(Çonti	nved)	•						
	63 .14		_							



LOG OF BORING NO. 8-180 OKLAUNION POWER STATION - UNITINO, 1 WILBARGER COUNTY TEXAS											
		~ + -				r, 1	57.4 N				
	PE.	301	RING: Undisturbed Sample L			Se . u	e Plo	<u>an di</u>	Bori	SHEAR STRENGTH SHE	
EPTH. FT	SYMBOL	SAMPLES	SOIL DESCRIPTION	CORE ECOVERY	OWS PER F	% PASSING 0. 200 SIEV	LIMIT	PLASTIC	I OISTURE		
-ª	1.5	Ŷ	ELEVATION: 1198.4	e e		a.Z			20		
			Hard brown clayey silt, -w/numerous calcareous nodules and fine roots								
- 5 -			-w/large calcareous nodules of 3.0' (ML) Hard reddish-brown silty clay, w/numerous weathered calcaroous	80		, 90	Эб	10	13		
			nodules -w/large calcareous nodules, coarse sand, and medium to 7								
			coarse gravel at 6.0' -w/coarse gravel at 8.0' (CL) Soft reddish-brown silty clay-		ma						
-15			stone Inclusions		44						
-20-				100			104				
			-w/occasional high angle fractures -w/bluish-gray clay seam at	100			1				
			24.5-25.5 ³ Hard reddish-brown silty clay		D⁄4"						
-30-			-w/occasional bluish-gray silty								
35	翻		Soft reddish-brown silty clay- stone, thin flat bedding								
<u>.</u>											

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	LOG OF BORING NO. 8-182 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS											
DEPTH. FT.	SYMBOL M	SAMPLES	SOIL DESCRIPTION	CORE 0	BLOWS PER FT. 10	% PASSING		PLASTIC 3	MOISTUNE 80	IN TONS/SOFT		
			Hard brown silty clay, w/fine roots, occasional coarse sand and weathered calcareous nodules (CL) Hard reddish-brown silty clay, w/numerous weathered calcar- eous nodules -extremely friable, w/fine grovel at 5.0 ¹ -w/some fine gravel and iron nodules at 7.0 ¹ -w/occasional bluish-gray silty clay inclusions at 10.0 ¹ -w/bluish-gray silty clay pockets (CL)	100	51/6 scat	99	40	20	12			
-15			Soft reddish-brown silty clay- stone, thin to medium bedding, w/numerous fine roots -continuing bluish-gray pockets	100	5)/5 seat	34	30	15				
			Very soft reddish-brown clayey shole -w/bluish-gray silty clay inclusions Soft reddish-brown silty clay- stone, thick bedded -slightly slickensided	100								
	(Continued)											

NESTRATIONAL SOR. SERVICES

NUMBER OF

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OF BORING NO. 3-182 (Contd.) LQG OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS i≽Ę, SHEAR STRENGTH ÷. SAMPLES DEPTH, FT MOISTURE CONTENT. PLASTIC LINUT % PASSIN NO. 200 SLE LINIT LINIT IN TONS/SQ FT SYMBOL ZU SU SU RECOVERN SOIL DESCRIPTION 1,5 Q.5 10 1 É -very soft clayey shale seam at 38.5-40.0 100 50 5.5 60 70 DEPTH TO WATER: Caved at 7.5 COMPLETION DEPTH: 40.0' DATE: 6/11/81 DATE: *5/5/*81 DI STE 114

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NATIONAL BOIL CENTRES, INC. . Computiting Shainsers.

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EPA ARCHIVE DOCUMENT

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AEPOKP000141

Γ	LOG OF BORING NO. B-186 OKLAUNION POWER STATION - UNIT NO. 1																													
							W	LEARGER	co	UNI	Ý, 1	ΓΕΧΑ	S																	
	YPI	Ē	<u>308</u>	ING:	Undist	urbed	Somp	la i		TION	t:Se	<u>e 21</u>	<u>20 01</u>	f Bor	ings, P	lote	3													
FUTH ET		SUL DESCRIPTION					CORE	OWS PER FT.	X PASSING 0. 200 SIEVE	LIQUID	PLASTIC LIMIT	I UISTURE	SHEAF IN TO	R S	stre /SQ	NGTH FT:	HT DRY WI													
			2	ELEVA	TION:	1202.	.7				° ž		[≥Ģ	<u>05</u>).; 1 : 1 :) TTTT	1.5	5-											
	2222			W/we	athered reddish	I calc	areous in silty	r accoules (CL) r clay	/																					
- 5				-w/n fissun -w/o and c	es and p ccasion corse g	ai cal ravel	ts at 4 logreo at 6.[nneu • 0° us nodules }'	100									8												
		L'HHH		clay i	inclusio	010 105	- 11-91 -	ay siny	t cx					-																
	Ĩ	X						(<u>C</u> L)		50 /9																				
-15	PURANA			Soft r thick -w/ac fractu	eddish- bedded :casionx res	al hig	n clays h angl	tone,	100					1 - - 																
-20	annann			-w/ac stone -w/ac	casione seams a casione	at blui mei ind al higi	ish-gra clusion hangi	ay clay - ns	100				: · ·			╉╌┦╸ ┥╿ ┽╷╄ ┽╷╄┽		┿╬┽╍┿ ╍┲╋╅╎┝ ┨╗┡╼┿┽												
-25		AN THAT		fine :	res grovel s	ed M c	at 26.(ינ		£7/4"					┿┥┿╂┤┆ ┽╎╎╎	· ↓ · ► · • · ↓ · ► · • ·	┝╬╬	╪ ╪ ╪ ╪ ╪ ╪ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇ ┇												
-30	ANNA	UNNNNN							100						╈															
		UUUUH	ļ.,	- 64			-1.				┉╍╡						┝╬┢╸													
-35	弦	11	د ۲ ۲ ۲ ۲	tons, ional ional i	aaish—c thick b bluish— <u>nelusio</u>	rown eddeo gray s 15	i, w/c iity cl																							
								(Contin	ued)				_																	
1073/			. 50K	. SERVICI	13.				-				•			2LAT	Ξ	19	PLATE 119											



	LOG OF BORING NO. B-190 OKLAUNION POWER STATION - UNIT NO. 1 WILBARGER COUNTY, TEXAS																		
	TΥ	۶E	80F	RING	<u>Undi</u> st	uiped :	Scmpla		Lọc,	ATIO	א: <u>5</u>	ee P	<u>lan (</u>	o <u>f 30</u>	rings,	Plat	a 3		
	DEPTH. FT	SYMBOL.	SOIL DESCRIPTION							OWS PER FT	% PASSING		PLASTIC	AOISTURE	SHE, IN	AR : TONS	STRE 7 SQ	NGTH FT:	NT DRY WT. BS./CU. FT.
ļ			¥	Hand	ATION: Idank b	1176. rcwn s	u ilty ele				1	<u>-</u>				<u>), t</u>)]]]]	1.9 () ()	
ļ				w/m	merous	fine n	no stoo	d 4	<u> </u>	<u> </u>		<u> </u>	<u> </u>						
	5			Hard W/co -w/co grave	raddisi Icareou alcareou alcareou alcareou alcareou	nodule: t=brow us nodu ous nodu 01	n silty des lules ar	(CL) clay, od fine	80		100	35	16	TO					
E				Soft:	raddish-	-brown	siltv o	i (CL)	00	+		-			┄┧╡╋┿╤┿┙ ┥╉╄┽┧	┝ ╤┿┝ ┤ ╺╷┊┊┨	┼ <u>┥┼</u> ┼┼┼	┿ ╋ ╋ ╋ ╋ ╋ ╋ ╋ ╋ ╋ ╋ ╋	
				stone ⊸w/c	lay bal	is				50/44						┥┥╺			
				Very shale	soft red , extrem	ldish-b nely si	town c ickensi	layey ded	100	•	96	36	20	18		70×10			
				Soft r stone, slicke	eddish- , thick nsided	brown beddeo	silty c 1, sligt	loy- htly	100	æ)/3"		5							
				Soft re slightl	eddish Iy slicki	brown ensided	clayey 1	shale,	100										
	5111			Soft re stone,	addish-d thick b	orown oodded	silty cl	مره											
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2009 DAM & DIKE INSPECTION REPORT WASTEWATER POND COMPLEX

GERS-09-025

OKLAUNION POWER PLANT

AMERICAN ELECTRIC POWER VERNON, TEXAS

PREPARED BY URS CORPORATION 9400 AMBERGLEN BLVD. AUSTIN, TEXAS 78717

June 2009

The technical material and data contained in this report were prepared under the supervision and direction of the undersigned, whose seal as Professional Engineer is affixed below:



Colin Young Texas License No. 81598 URS Corporation Texas Firm No. 3162

ANNUAL DAM AND DIKE INSPECTION REPORT OKLAUNION POWER STATION WASTEWATER POND COMPLEX YEAR 2009 GERS-09-025

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APPENDIX A: VICINITY MAP AND PLAN VIEW

APPENDIX B: INSPECTION PHOTOGRAPHS

INTRODUCTION

AEPSC (American Electric Power Service Corporation) Civil Engineering manages the Dam and Dike Inspection and Maintenance Program (DIMP) at AEP facilities. As part of DIMP, staff from the Geotechnical Engineering Section conducts dam and dike inspections annually. The 2009 inspection of the wastewater pond complex dikes at the Oklaunion Power Plant was performed by URS Corporation. Messrs Colin Young, P.E. and Lance Finnefrock were accompanied by Messrs. William Smith, P.E., and Kenneth Patton, P.E. of AEP. This report was prepared by Mr. Colin Young, P.E. and serves as a summary of the inspection and an assessment of the general conditions of the facility.

Mr. Kenneth Patton, P.E. of AEP Plant Engineering Region 5, was the facility contact for the inspection. The inspection was performed on April 2, 2009. Weather conditions were very windy, with temperatures in the 50's and 60's.

General Information

The Oklaunion Power Plant is owned by American Electric Power and is located at 12567 FM Rd 3430, Vernon, TX 76384. The plant is a coal-fired facility, which features a number of wastewater evaporation ponds containing cooling tower blowdown. A few of the wastewater ponds also contain some byproducts of the coal combustion process. Earthen fill dikes retain the waste until it is sufficiently dry to be hauled away and landfilled. The plant has 11 wastewater evaporation ponds, with a total area of 335.9 acres. Two proposed evaporation ponds at the northwest corner of the site would have a total area of 54.6 acres. In addition, there is one make-up water supply pond (52.8 ac.), a coal pile runotT pond (27.3 ac.), and a wastewater/sludge pond (22.6 ac.).

SUMMARY OF VISUAL OBSERVATIONS

The summary of the visual observations presented herein uses terms to describe the general appearance or condition of an observed item, activity or structure. Their meaning is understood as follows:

- Good: A condition or activity that is generally better or slightly better than what is minimally expected or anticipated from a design or maintenance point of view.
- Fair or A condition or activity that generally meets what is minimally satisfactory: expected or anticipated from a design or maintenance point of view
- Poor: A condition or activity that is generally below what is minimally expected or anticipated from a design or maintenance point of view.
- Minor: A reference to an observed item (e.g., erosion, seepage, vegetation, cracks, concrete surface etc.) where the current maintenance condition is below what is normal or desired, but which is not currently causing concern from a structure safety or stability point of view
- Significant: A reference to an observed item (e.g. erosion, seepage, vegetation, cracks, concrete surface etc.) where the current maintenance program has neglected to improve the condition. Usually, conditions that have been identified in previous inspections, but have not been corrected.
- Excessive. A reference to an observed item (e.g., erosion, seepage, vegetation, cracks, concrete surface etc.) where the current maintenance condition is above or worse than what is normal or desired, and which may have affected the ability of the observer to properly evaluate the structure or particular area being observed or which may be a concern from a structure safety or stability point of view.

Results of the visual inspection performed on April 2, 2009 are summarized below Pond water elevation is presented in the instrumentation data section of this report. The inspection focused on the exterior western and southern dikes, as these were the only locations where dike failure could lead to uncontrolled release of impounded water off-site

Outfall Spillway

The outfall spillway is located on the south slope of Wastewater Evaporation Pond #7, and runs adjacent to the east slope of Pond #6. Brush and trees were recently cleared from the channel area, leaving behind disturbed soil and minimal vegetative cover. Several small

trees remain in the concrete-lined spillway chute itself, situated on the south downstream slope of Pond #7.

Wastewater Evaporation Pond #6

Pond #6 is located at the south-central edge of the site. No signs of slope failure, slumping, or scepage were observed on the downstream slope. Vegetative cover on the east downstream slope of Pond #6 is very poor due to recent tree clearing. At the southwest corner of the pond, vegetative cover is also very poor and excessive crosion gullies (up to 2 feet deep) (Photographs 2-6) have developed on the downstream slope extending from the crest. Aside from the potential threat of headcutting, the crest of the dam appeared to be in good condition with no unusual deformation or misalignment.

Wastewater Evaporation Pond #7

Pond #7 is located on the southeastern corner of the site. The crest of the dam appeared to be in good condition with no unusual deformation or misalignment. No evidence of slope failure, slumping, or seepage was observed on the downstream slope (Photograph 1) Vegetative cover on the south downstream slope is fair to poor. As previously mentioned, there are trees growing in the outfall spillway on the downstream slope. Wave action has resulted in significant erosion on its upstream slope, with near vertical scarps observed a short distance above the break height of the waves.

Wastewater Evaporation Pond #8

Pond #8 is located near the western boundary of the site and adjacent to Pond #9. There were no observed signs of slope failure, slumping, or seepage on the downstream slope Vegetative cover ranges from fair to very poor, and minor to significant erosion is widespread. Areas of excessive erosion (Photographs 11-14) are found at the northwest corner of the pond, where erosion gullies, scarps, and sloughs are present on the downstream slope. Additionally, channelization due to stormwater runoff at the north of the pond is starting to cut away at the toe of the slope, which could eventually lead to stability concerns if left unaddressed. The crest did not exhibit any unusual deformation or misalignment, but erosion rills and gullies have begun headcutting at the downstream edge of the crest.

Wastewater Evaporation Pond #9

Pond #9 is located near the southwest corner of the site and adjacent to Pond #10. No evidence of slope failure, slumping, or seepage was observed on the downstream slope. The quality of vegetative cover on the downstream slopes of Pond #9 generally ranges from fair to poor. However, the occurrence of minor to significant erosion rills and gullies was widespread on these slopes (Photograph 7), even where vegetation would be considered

acceptable. Erosion is excessive in localized areas of very poor vegetative cover, including slough (shallow slide) areas and gullies up to 1 foot deep. The crest did not exhibit any unusual deformation or misalignment, but erosion rills and gullies have begun headcutting at the downstream edge of the crest.

Wastewater Evaporation Pond #10

Most of Pond #10 is an at-grade impoundment; the southeast corner is the only location at which there is a dike. Vegetative cover on the downstream slope is generally poor, with minor erosion rills. No evidence of slope failure, slomping, or seepage was observed on the downstream slope, and the crest is in good condition.

CONCLUSIONS AND RECOMMENDATIONS

There was no evidence of significant distress that would indicate immediate concern regarding the integrity of the wastewater pond dikes. As such, it is concluded that the dikes are performing as designed with respect to water retention, but not with respect to erosion control. It appears that a widespread crossion problem is negatively affecting the exterior slope of the main containment dike around ponds 6 through 10. Several items should be addressed in order to mitigate/prevent future or current progressive deficiencies. The recommendations presented below address those items as well as the continued care and routine maintenance of the facility:

- Any erosion gullies and slough areas that are encountered should be stabilized as soon as possible. Erosion gullies and slough areas may be stabilized by re-dressing the slope and placing riprap or revegetating depending on the slope gradients and propensity for concentrated water flow.
- 2. The upstream erosion at Pond #7 should be mitigated as soon as possible, and can be achieved either by placing rip rap on the slope or by constructing a wall to dissipate wave energy.
- 3. Vegetation should be established on the bare slopes of Wastewater Evaporation Ponds # 6, 8, 9, and 10. The soils in the area are highly erosive as evidenced by the deep gullies. The U.S. Dept. of Agriculture Natural Resource Conservation Service (NRCS) office can provide suitable seed mix design for the local climate and soils.
- 4. The earth fill slopes of the dikes should be maintained free of excessive brush and woody vegetation. It appears that AEP has been diligent in these efforts, however, it is important to establish a grass cover as soon as possible after disturbing the ground for brush cutting activities. Grassed areas should be mowed regularly to prevent the growth of woody vegetation.
- 5. The small trees near the toe of the outfall spillway at Pond #7 should be removed.

- 6. Monitoring procedures and maintenance activities should be implemented in coordination with the AEP Geotechnical Engineering Group.
- 7. If allowable under state environmental regulation, URS recommends that AEP consider utilizing some of the water that would otherwise go to evaporation ponds to irrigate the slopes of the dikes. This would promote the growth of healthy grass ground cover, thus reducing future crosion damage. If implemented, it should be done after the presently eroded areas are repaired and re-seeded.

Based on the inspection and review of relevant documents, URS Corporation believes that the Oklaunion Power Station wastewater ponds have a general satisfactory appearance and are currently in satisfactory condition. Inspections and monitoring should continue If you have any questions with regard to this report, please do not hesitate to contact Mr. Colin J. Young, P.E. at 512,419,5903.

APPENDIX A

VICINITY MAP AND PLAN VIEW







APPENDIX B

INSPECTION PHOTOGRAPHS



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Photo # 4	
South crest and	
downstream slope of	
Pond #6. looking east.	
Photo # 5	
Erosion gullies on	
downstream slope at	
southwest corner of Pond	
#6.	



Photo # 8	
West downstream slope on southern end of Wastewater Evaporation Pond #8. Notice erosion gullies.	
Photo # 9	
Erosion scarp on west embankment at southern end of Pond #8.	
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Photo # 12

Close up view of erosion slough on west embankment at northern end of Pond #8.



Photo #13

Erosion scarp on west embankment at northern end of Pond #8.



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Photo #14

Erosion channel at northern toe of embankment for Pond #8.



AEPOKP000095

DAM & DIKE INSPECTION REPORT

OKLAUNION POWER STATION WASTEWATER POND COMPLEX VERNON, TX

INSPECTION DATE August 4, 2010

PREPARED BY

DATE <u>10 29-101</u>0 William R. Smith, P.E.

REVIEWED BY Mary F

Gary F. Zych, P.E.

APPROVED BY

DATE 11/2/2012 1 Barch Gary F. Zych, P.E. Manager – Geotechnical Engineering



DATE 11/2/2010

PROFESSIONAL ENGINEER SEAL & SIGNATURE

DAM AND DIKE INSPECTION REPORT OKLAUNION POWER STATION WASTEWATER POND COMPLEX YEAR 2010 GERS-10-040

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Wastewater Evaporation Pond #95
Wastewater Evaporation Pond #105
Wastewater Evaporation Pond #125
Wastewater Evaporation Pond #13
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APPENDIX A: VICINITY MAP AND PLAN VIEW APPENDIX B: INSPECTION PHOTOGRAPHS

INTRODUCTION

American Electric Power Service Corporation's (AEPSC) Civil Engineering Division administers the Oklaunion Power Station Dam Inspection and Maintenance Program (DIMP). As part of the DIMP, staff from the Geotechnical Engineering Services Section periodically conducts dam and dike inspections. The 2010 inspection of the wastewater pond complex dikes at Oklaunion Station was performed by Mr. William R. Smith, P.E. of AEPSC Civil Engineering. The report presents a summary of the inspection and an assessment of the condition of the facilities.

The inspection was performed on August 4, 2010. Mr. Steve Lewis, maintenance supervisor at the Oklaunion Plant, was the facility contact and provided access and coordination activities. Weather conditions were clear with good visibility, winds were from the sonthwest at approximately 1 to 5 mph and temperatures ranging between 90 and 95°F. Inspection observations were briefly discussed with Mr. Lewis after completion of the field work.

General Information

The Oklaunion Power Station is owned by American Electric Power and is located at 12567 FM Rd 3430, Vernon, TX 76384. The plant is a coal-fired facility, which includes a number of wastewater evaporation ponds containing cooling tower blowdown. A few of the wastewater ponds also contain some by-products of the coal combustion process.

Earth-fill dikes retain plant wastes until they are sufficiently dry to be hauled away and landfilled. The plant has 13 wastewater evaporation ponds, with a total area of 385.9 acres. In addition, there is one make-up water supply pond (52.8 ac.), a coal pile ranoff pond (27.3 ac.), and a wastewater/sludge pond (22.6 ac.). A facility map and plan view are provided in Appendix A.

For reference purposes in this report, the main evaporation pond complex area refers to the group of ponds located on the south side of the plant access road. Two recently constructed diked ponds, #12 and #13, are situated north of the plant access road separately from the main evaporation pond complex area.

SUMMARY OF VISUAL OBSERVATIONS

The summary of the visual observations presented herein uses terms to describe the general appearance or condition of an observed item, activity or structure. The meaning of these terms is understood as follows:

- Good: A condition or activity that is generally better or slightly better than what is minimally expected or anticipated from a design or maintenance point of view.
- Fair/Satisfactory: A condition or activity that generally meets what is minimally expected or anticipated from a design or maintenance point of view.
- Poor: A condition or activity that is generally below what is minimally expected or anticipated from a design or maintenance point of view.
- Minor: A reference to an observed item (e.g. erosion, seepage, vegetation, etc.) where the current maintenance condition is below what is normal or desired, but which is not currently causing concern from a structure safety or stability point of view.
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- Excessive: A reference to an observed item (e.g., erosion, seepage, vegetation, eracks, concrete surface etc.) where the current maintenance condition is above or worse than what is normal or desired, and which may have affected the ability of the observer to properly evaluate the structure or particular area being observed or which may be a concern from a structure safety or subility point of view.

Results of the visual inspection performed on August 4, 2010, are summarized below, with inspection photographs included as Appendix B. The inspection focused on the exterior western and southern dikes, impounding the main complex area, and the dikes enclosing the newly constructed Ponds #12 and #13 as these were the only locations where dike failure could lead to incontrolled release of impounded water off-site.

Outfall Spillway

The outfall spillway is located on the south dike of Wastewater Evaporation Pond #7, and the discharge channel runs adjacent to the east dike of Pond #6. The spillway has been

blocked by a concrete wall, approximately 4 ft, in height, built across the spillway channel at the control section.

The spillway control section is in generally in poor condition with extensive cracking of concrete elements. A general view of the spillway control section is provided in Photo 1 and a closer view showing the extensive cracking of the concrete is shown in Photo 2, taken in March 2010. The shore protection revetment of stacked, sand-cement mix paper bags, shown in Photo 1, was added in front of the spillway control section and extending along the shoreline beyond the spillway in both the left and right directions in 2009. This revetment was added to provide wave crossion protection and to replace the front portion of the control section that had previously been undermined from wave erosion and broken off as shown in Photo 3, taken in May 2008.

The spillway discharge channel was in generally fair condition. All brush and trees noted in the downstream channel during the 2009 inspection had been cleared. However, the channel bed was barren of a protective grass cover because of the recent clearing as depicted in Photo 4.

Wastewater Evaporation Pond #1

Pond #1 is located at the northwest corner of the main evaporation pond complex area. The upstream slope of this embankment was under construction, being relined with a compacted elay and HDPE composite liner system, at the time of inspection. As a consequence, the crest and upstream slope were not observed,

The downstream slope was in generally fair condition. There were no observed signs of slope failure, slumping, or scepage on the slope. Vegetative cover was well-established but about 2 feet in height precluding a thorough inspection of the slope. Significant erosion from stormwater runoff was observed to have cut the toe of the slope along the entire slope. This is shown in Photo 5. This could eventually lead to stability concerns if left unaddressed.

Wastewater Evaporation Pond #6

Pond #6 is located at the south-central edge of the main evaporation pond complex area. The upstream slope has been buried with solid waste materials to the crest and is not visible as shown in Photos 6 and 7. The crest of the dam appeared to be in good condition with no unusual cracking, rutting, settlement, deformation, or misalignment. Photos 6 and 7 provide views of the crest.

The downstream slope was in overall fair condition. No signs of slope failure, slumping, or scepage were observed on the downstream slope. However, vegetative cover on the downstream (east and south) slopes of Pord #6 was in fair to poor condition at the time of inspection. The slopes had been seeded with pative, peremial grasses in March 2010, shown in Photo 8, but the vegetation at the time of inspection consisted of dead grasses and live weeds 2 to 3 feet in height as shown in Photos 9 and 10. No erosion gulfies, as

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observed during the 2009 inspection, were noted on the downstream slopes; however, the excessively high vegetation made observation of the entire slope very difficult.

Wastewater Evaporation Pond #7

Pond #7 is located on the southeast corner of the main evaporation pond complex area. The upstream slope was in generally fair to poor condition. As noted in the Outfall Spillway section of this report, shore protection revetment in the form of stacked, sandcement mix paper bags was added in front of the spillway control section that extends along the shoreline beyond the spillway in both the left and right directions as shown in Photo 1. This revetment was added to replace the failed front section of the spillway control section and to provide wave crossion protection for the upstream slope in the area. The stacked-bag revetment itself was in good condition and is providing shore protection within the armored area. However, the soil shoreline immediately beyond each end of the revetment was significantly eroded with embankment material sloughing as shown in Photos 11 through 13. The slope area above the revetment was very sparsely vegetated or barren and had many crossion rills developing as shown in Photo 13.

The downstream slope was in generally fair condition. No evidence of erosion, slope failure, slumping, or scepage was observed on the slope. However, vegetative cover on the slope was 2 to 3 feet in height and sparse in some areas at the time of inspection as shown in Photos 14 and 15. The crest of the dam appeared to be in good condition with no unusual cracking, rutting, settlement, deformation, or misalignment.

Wastewater Evaporation Pond #8

Pond #8 is located near the western boundary of the main evaporation pond complex area, adjacent to and immediately north of Pond #9. The upstream slope of this embankment is covered with a high-density polyethylene (FIDPE) liner and was not directly observable. No undulations, bulges, depressions, or undue stresses on the liner were observed. The crest of the embankment appeared to be in fair condition with no unusual cracking, settlement, deformation, or misalignment. However, at this location the crest has no surface cover of gravel to provide protection from rutting due to vehicular traffic and there is very little, if any, access to the area by vehicle during inclement weather conditions because of the slippery road surface.

The downstream slope was in generally fair condition. There were no observed signs of slope failure, slumping, or scepage on the slope. Vegetative cover ranged from fair to poor with very similar features along the central and southern portions of the slope as those noted for the downstream slope at Pond #6 (recent seeding but dead covergent grasses and live weeds 2 to 3 feet in height). A typical view of the southern portion of the slope is provided in Photo 16. Along the northern portion of the slope, erosion rills have begun to develop in a sparsely vegetated area as shown in Photo 17. An area of significant erosion at the top of the slope due to stormwater runoff was observed at the northern end of the pond as shown in Photo 18. This could eventually lead to stability concerns if left unaddressed.

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Wastewater Evaporation Pond #9

Pond #9 is located near the southwest corner of the mein evaporation pond complex area, adjacent to and immediately north of Pond #10. The opstream slope of this embankment is covered with an HDPE liner and was not directly observable. No undulations, bulges, depressions, or undue stresses on the liner were observed. The crest of the embankment appeared to be in fair condition with no unusual cracking, rutting, settlement, deformation, or misalignment. However, at this location the crest has no surface cover of gravel to provide protection from rutting due to vehicular traffic and there is very little, if any, access to the area by vehicle during inclement weather conditions because of the slippery read surface.

The downstream slope was in generally fair condition. There were no observed signs of seepage on the slope. Vegetative cover ranged from fair to poor with very similar features as those noted for the downstream slopes at Ponds #6 and #8 (recent seeding but dead emergent grasses and live weeds 2 to 3 feet in height). A typical view of the slope is provided in Photo 19. One area of sparse vegetation and incipient slumping was noted at the northern end of the slope near the southern border of adjacent Pond #8. This area is depicted in Photo 20.

Wastewater Evaporation Pond #10

Pond #10 is located at the southwest corner of the main evaporation pond complex area. The upstream slope of this embankment is covered with an HDPE liner and was not directly observable. No undulations, bulges, depressions, or undue stresses on the liner were observed. The crest of the embankment appeared to be in fair condition with no unusual cracking, rutting, settlement, deformation, or misalignment. However, at this location the crest has no surface cover of gravel to provide protection from rutting due to vehicular traffic and there is very little, if any, access to the area by vehicle during inelement weather conditions because of the slippery road surface. A view of the crest from the southwest corner of the pond, looking north is provided in Photo 21.

The downstream slope was in generally fair condition. No evidence of crusion, slope failure, slumping, or scepage was observed on the slope. Vegetative cover ranged from fair to poor with very similar features as those noted for the downstream slopes at Ponds #6, #8, and #9 (recent seeding but dead emergent grasses and live weeds 2 to 3 feet in height). A typical view of the slope is provided in Photo 22.

Wastewater Evaporation Pond #12

Pond #12 is situated north of the plant access road at a location separate from the main evaporation pond complex area. The pond was constructed during the latter part of 2009. The upstream slope of this embankment is covered with an HDPE liner and was not directly observable. No undulations, bulges, depressions, or undue stresses on the liner were observed. The crest of the embankment appeared to be in good condition with nounusual cracking, rutting, settlement, deformation, or misalignment.

The downstream slope was in generally fair condition. No evidence of erosion slope failure, slumping, or scepage was observed on the slope. Vegetative cover ranged from fair to poor. A view of the south downstream slope, with the vegetation in fair condition, is provided as Photo 23. A view of the north downstream slope and crest, with the vegetation on the slope in poor condition, is provided as Photo 24.

Wastewater Evaporation Pond #13

Pond #13 is situated north of the plant access road at a location separate from the main evaporation pond complex area. The pond was constructed in 2010 with construction complete prior to the date of inspection. The upstream slope of this embankment is covered with an HDPE liner and was not directly observable. No undulations, bulges, depressions, or undue stresses on the liner were observed. The crest of the embankment appeared to be in good condition with no unusual cracking, rutting, settlement, deformation, or misalignment. A view of the crest, in good condition, is provided as Photo 25.

The downstream slope was in generally good condition. No evidence of erosion, slope failure, slumping, or seepage was observed on the slope. Vegetative cover was generally good with some minor areas of sparse vegetation. Vegetation height was about 4 to 5 inches. A view of the south downstream slope is provided as Photo 26.

CONCLUSIONS AND RECOMMENDATIONS

Based on the visual observations during the dike inspection at Oklaunion Station, the dikes containing the main evaporation pond complex area and Pond #12 are generally in fair condition, and the dikes containing Pond #13 are in good condition. A summary of our conclusions and recommendations for general maintenance and continued monitoring, as well as any recommendations for remedial activities, is provided as follows.

There was no evidence of significant distress that would indicate immediate concern regarding the integrity of the wastewater pond dikes. As such, it is concluded that the dikes are performing as designed. There were significant areas where the establishment and control of vegetation has been somewhat problematic, but the widespread crosion problems over exterior slope of the main containment dike around ponds 6 through 10 noted in the 2009 report had been addressed through an extensive program of regrading and seeding with native perennial grasses. Recent communication with the plant, since the date of inspection, indicates that areas where dead grasses were observed during the inspection are re-emerging with native grass species that were in the original seed mix, and that the dead grasses referred to in this report were from a species in the mix that was F

used to establish a quick first-year cover that was stressed by the dry conditions prior to the inspection.

Recommendations for General Maintenance and Monitoring Activities

- Any crosion gullies and slough areas that are encountered should be stabilized as soon as possible. Brosion gullies and slough areas may be stabilized by redressing the slope and placing riprap or revegetating depending on the slope gradients and propensity for concentrated water flow.
- The earth fill slopes of the dikes should be maintained free of excessive brush and woody vegetation. It is important to establish a grass cover as soon as possible after disturbing the ground for brush cutting activities.
- Grassed areas should be mowed regularly. Mowing at least twice per year and maintaining vegetation height at a maximum of 6 to 8 inches is recommended. High vegetation obstructs visual observations during inspection, encourages burrawing animal activity, and thins the ground cover by decreasing vegetation stem density and encouraging the growth of annual grasses and other invasive weeds. Any areas that are not accessible to mowing equipment should be controlled by the use of weed trimmers, power brush-cutters, or other suitable vegetation control processes.
- Any animal borrows on the dikes should be filled with an impervious meterial or cementious grout, dressed with topsoil, and regraded followed by seeding any disturbed ground with native grasses to establish a complete ground cover for crosion protection.
- Monitoring procedures, maintenance activities, and reporting should be implemented in coordination with AEP Civil Engineering.

Recommendations for Specific Remedial Activities

- Shore protection beyond the ends of the revetment around the split/way at Wastewater Pond #7 should be added to prevent further erosion of the dike at these two locations. Extension of the revetment is not recommended since this would likely transfer the problem to the ends of the new revetment. We recommend regrading to a uniform slope and installing riprap shore protection over a geotextile or gravel filter layer.
- An engineering analysis should be performed to verify that the blocking of the spillway at Wastewater Poud #7 will not cause the dikes to be overtopped during a severe storm if this has not been performed.
- The discharge channel at the spillway at Wastewater Pond 47 should be seeded with native, non-invasive perennial grasses for the entire length where it is

adjacent to the east dike of Wastewater Pond #7 to suppress the growth of woody vegetation, invasive weeds and annual grasses. The barren upstream slope at Pond #7, extending about 250 feet left of the spillway where the embankment meets natural ground and about 150 feet to the right of the spillway, should be regraded and similarly seeded to prevent further development of erosion tills.

Erosional features identified in this report, specifically the cutting away at the toes
of the slopes at Ponds #1 and #8, and the incipient slumping at the north end of
Pond #9 should be regraded to a uniform slope, with any eroded material replaced
with suitable compacted till, and seeded with native, non-invasive perennial
grasses to suppress the growth of woody vegetation, invasive weeds and annual
grasses.

If you have any questions with regard to this report, please do not hesitate to contact Mr. William R. Smith, P.E. at 614-716-2906 (Audinet: 200-2906) or Gary Zych, P.E. at 614-716-2917 (Audinet: 200-2917).

APPENDIX A

VICINITY MAP AND PLAN VIEW

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US EPA ARCHIVE DOCUMENT



APPENDIX B

INSPECTION PHOTOGRAPHS

APPENDIX B OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 1

General view of Pond #7 spillway area showing the control section blocked by a concrete wall and recently installed shore protection revetment, in front, in good condition but erosion progressing beyond revetment.



Pond #7 photo taken in March 2010 showing spillway control section in generally poor condition with extensively cracked concrete elements. The concrete wall blocking potential flow through the spillway is at the left in the photo.



Photo 3

Pond #7 photo taken in May 2008 showing failed front of spillway controlsection that was replaced in 2009 with the revetment shown in Photo 1.

APPENDIX 3 OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 4

The discharge channel downstream of the spillway was cleared of woody vegetation and brush. However, the channel was barren.

Photo 5

Typical view of downstream slope at Pond #1 looking north. Note significant erosion cutting along the toc of the slope.

Photo 6

Crest and buried upstream slope of Pond #6 looking west from near the southeast corner of the pond.

APPENDIX B OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS

Photo 7



Crest and baried upstream slope of Pond-#6 looking east from near the southwest corner of the pond.

Photo 8

Downstream slope of east dike at Pond #6 in March 2010 seeded with native, perennial grasses.



Photo 9

Downstream slope of east dike at Pond-#6 at time of inspection with vegetative cover of dead grasses and live weeds 2 to 3 feet in height.



APPENDIX B OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 10

Downstream slope of south dike at Pond #6 at time of inspection with vegetative cover of dead grasses and live weeds 2 to 3 feet in height. The biodegradable cord from recent seed matting installation is visible in the photo foreground.

Photo 11

Pond #7 upstream slope at spillway area with significant erosion progressing around the western end of the shore protection revetment. The western end of the revetment is at the bottom right in the photo.

Photo 12

Pond #7 upstream slope immediately east of the revetment-reinforced spillway area, looking west, with significant wave erosion progression and barren slope.

APPENDIX 5 OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 13

Poud #7 upstream slope area above the cast end of the shore protection revetment around the spillway, looking cast, with many crosion rills forming on the barren ground and embankment erosion beyond the end of the revetment (opposite view from Photo 12).

Photo 14

Downstream slope at Pond #7 with grass cover at 2 to 3 feet in height over most of the slope and some sparsely vegetated areas with a cover of broomweed and other native weedy vegetation.

Photo 15

Downstream slope at Pond #7 near spillway discharge channel with grass cover at 2 to 3 feer in height and sparsely vegetated areas with a cover of broomweed and other native weedy vegetation.



APPENOIX 8 OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 16

Southern portion of downstream slope at Pond $\pi 8$ with vegetative cover of dead grasses and live weeds 2 to 3 feet in height.



Northern portion of downstream slope at Pond 48 with dovelopment of erosion rills at a barren area.

Photo 18

Significant erosion cutting at the toe of the north end of the downstream slope at Pond #8.



APPENOIX IS OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 19

Downstream slope of west dike at Pond #9 with vegetative enver of dead grasses and live weeds 2 to 3 feet in height along with some sparsely vegetated areas.





Sparsely vegetated area with incipient slumping at the north end of the western dike at Pond 49, looking north.

Photo 21

View of crest of western dike from southwest corner of Pond #10 looking north. The crest along the west side of Ponds #8, #9, and #10 has no surface cover of gravel.

APPENDIX * OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 22

Downstream slope of west dike at Pond #10 with vegetative cover of dead grasses and live weeds 2 to 3 feet in height and a sparsely vegetated area in the photo foreground.

Photo 23

Downstream slope of south dike at Pond #12 with recently planted vegetation in fair condition.

Photo 24

Downstream slope and crest of north dike at Pond #12 with recently planted vegetation in poor condition and crest in good condition.

APPENDIX D OKLAUNION WASTEWATER POND COMPLEX DIKE INSPECTION PHOTOGRAPHS



Photo 25

Crest of south dike at Pond #43 in good condition.

Photo 26

Downstream slope of south dike at Pond #13 with recently planted vegetation in good condition.

Appendix A: Inspection Report

Inspection Results-Dam Conditions

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Guidelines for Operation and Maintenance of Danis in Texas

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Guidelines for Operation and Maintenance of Dans in Texas

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TEXAS COMMISSION ON ENVIRONMENTAL QUALITY

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Guidelines for Operation and Maintenance of Dema in Texas

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Appendix A: Inspection Report

Inspection Results—Dam Conditions

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Guideilnes for Operation and Maintenance of Dams in Texas

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Guidelines for Operation and Maintenance of Dams in Texas

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Suldelines for Operation and Maintenance of Dams in Texas

Inspection Monitoring Form

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	TEXAS COMMISSION ON ENVIRONMENTAL OUALITY

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Site Name:	OKLAUNION POWER STATION	Date:	February 23, 2011
Unit Name:	WASTERWATER EVAPORATION POND 6	Operator's Name:	AMERICAN ELECTRIC POWER
Unit I.D.:	WMU 001	Hazard Potential Classification:	High 🗌 Significant 🗌 Low 🔀
	Inspector's Name:	Kyle Shepard & Andy Cueto	

Check the appropriate box below. Provide comments when appropriate. If not applicable or not available, record "N/A". Any unusual conditions or construction practices that should be noted in the comments section. For large diked embankments, separate checklists may be used for different embankment areas. If separate forms are used, identify approximate area that the form applies to in comments.

		-			
	Yes	No		Yes	No
1. Frequency of Company's Dam Inspections?	Х		18. Sloughing or bulging on slopes?		Х
2. Pool elevation (operator records)?	Х		19. Major erosion or slope deterioration?		Х
3. Decant inlet elevation (operator records)?	Х		20. Decant Pipes:		
4. Open channel spillway elevation (operator records)?	N/A	N/A	Is water entering inlet, but not exiting outlet?	N/A	N/A
5. Lowest dam crest elevation (operator records)?	Х		Is water exiting outlet, but not entering inlet?	N/A	N/A
If instrumentation is present, are readings recorded (operator records)?		Х	Is water exiting outlet flowing clear?	N/A	N/A
7. Is the embankment currently under construction?		х	21. Seepage (specify location, if seepage carries fines, and approximate seepage rate below):		
8. Foundation preparation (remove vegetation, stumps, topsoil in area where embankment fill will be placed)?	N/A	N/A	From underdrain?		Х
9. Trees growing on embankment? (If so, indicate largest diameter below)		Х	At isolated points on embankment slopes?		Х
10. Cracks or scarps on crest?		Х	At natural hillside in the embankment area?		Х
11. Is there significant settlement along the crest?		Х	Over widespread areas?		Х
12. Are decant trashracks clear and in place?	N/A	N/A	From downstream foundation area?		Х
13. Depressions or sinkholes in tailings surface or whirlpool in the pool area?		Х	"Boils" beneath stream or ponded water?		Х
14. Clogged spillways, groin or diversion ditches?	N/A	N/A	Around the outside of the decant pipe?		Х
15. Are spillway or ditch linings deteriorated?	N/A	N/A	22. Surface movements in valley bottom or on hillside?		Х
16. Are outlets of decant or underdrains blocked?	N/A	N/A	23. Water against downstream toe?	Х	
17. Cracks or scarps on slopes?		Х	24. Were Photos taken during the dam inspection?	Х	

Major adverse changes in these items could cause instability and should be reported for further evaluation. Adverse conditions noted in these items should normally be described (extent, location, volume, etc.) in the space below and on the back of this sheet.

 Issue #
 Comments

 All active ponds on this site were incised. One pond that has not been active for 3 years was not incised but was in the process of being closed.

1



Coal Combustion Waste (CCW)

Impoundment Inspection

Impoundment NPDES Permit	TX0087815		INSPECTOR			
Date Impoundment Name	11.16.2007 WASTEWAT	ER EVAPOR	ATION POND NO	. 6		
Impoundment Company EPA Region	AMERICAN REGION 6	ELECTRIC PC	OWER			
State Agency (Field Office) Address Name of Impoundment	1977 INDUS WASTEWAT	STRIAL BOUL	EVARD, ALBILEN ATION POND NO	E, TEXAS 796 . 6	02-7833	
(Report each impoundme	ent on a sepai	rate form un	nder the same Im	poundment l	NPDES Permit r	umber)
New	Update 🛛	\triangleleft		Yos		No
Is impoundi Is water or ccw currently beir IMPOUNDMENT FUN	ment current ng pumped ir NCTION: W/	l y under co n nto the impo ASTEWATER	nstruction? oundment? AND RESIDUAL S		VAPORATION P	
Nearest Downstream Towr	n Name: NC) TOWN WIT	THIN A 18 MILE R	ADIUS		
Distance from the impour	ndment: ±1	8 MILES				
Location: Latitude 34 I	Degrees	04	Minutes	47.99	Seconds	N
Longitude 99 I	Degrees	10	Minutes	45.00	Seconds	w
State			County			
Does a state a	gency regula	ite this impo	oundment?	Yes		No
	If Se	o Which Sta	te Agency?			



HAZARD POTENTIAL (In the event the impoundment should fail, the following would occur):

- LESS THAN LOW HAZARD POTENTIAL: Failure or misoperation of the dam results in no probable loss of human life or economic or environmental losses.
- **LOW HAZARD POTENTIAL:** Dams assigned the low hazard potential classification are those where failure or misoperation results in no probable loss of human life and low economic and/or environmental losses. Losses are principally limited to the owner's property.
 - **SIGNIFICANT HAZARD POTENTIAL:** Dams assigned the significant hazard potential classification are those dams where failure or misoperation results in no probable loss of human life but can cause economic loss, environmental damage, disruption of lifeline facilities, or can impact other concerns. Significant hazard potential classification dams are often located in predominantly rural or agricultural areas but could be located in areas with population and significant infrastructure.
 - **HIGH HAZARD POTENTIAL:** Dams assigned the high hazard potential classification are those where failure or misoperation will probably cause loss of human life.

DESCRIBE REASONING FOR HAZARD RATING CHOSEN:

UPON VISUAL INSPECTION, THERE WAS NO SIGHT OF ANY POSSIBLE BREACHING OR OVERTOPPING OF THIS IMPOUNDMENT.



CONFIGURATION:





TYPE OF OUTLET (Mark all that apply)



The Impoundment was Designed By **TIPPETT & GEE**:



6

	Yes	No
Has there ever been a failure at this site?		\square

If So When?

If So Please Describe :



7

	Yes	No
Has there ever been significant seepages at this site?		\boxtimes

If So When?

If So Please Describe :

8

	Yes	No
Has there ever been any measures undertaken to monitor/lower Phreatic water table levels based		
on past seepages or breaches at this site?		\square
If so, which method (e.g., piezometers, gw		

pumping,...)?

If So Please Describe :



ADDITIONAL INSPECTION QUESTIONS

Concerning the embankment foundation, was the embankment construction built over wet ash, slag, or other unsuitable materials? If there is no information just note that.

NO

Did the dam assessor meet with, or have documentation from, the design Engineer-of-Record concerning the foundation preparation?

YES

From the site visit or from photographic documentation, was there evidence of prior releases, failures, or patchwork on the dikes?

NO

9

Available Information Checklist Coal Combustion Residuals Impoundment (CCRI) Dam					
		PROVIDED BY UTILITY			
	YES	NO	N/A		
1. Descriptive Information:					
a) Impoundment Capacity (Normal & Max)	x				
b) Impoundment Surface Area	x				
c) Hazard Classification	х				
d) Freeboard (Normal & Min)	x				
e) Maximum Dam Height	x				
f) Dam Crest Elevation	х				
g) Crest Width	x				
h) Upstream Slope Inclination	x				
i) Downstream Slope Inclination	x				
j) Spillway Type, Size, & Crest Elevation			х		
k) Outlet Condit Type, Size, & Max Flow Capacity			х		
I) Historical Maximum Pond Elevation	x				
m) Year Built	х				
n) Design Life	х				
o) Specific Wastes Permitted in Impoundment	х				
p) Other (describe)					
2. Regional Map showing CCWI & schools, hospitals, etc. w/i 5 mi downgradient		x			
3. Management Unit Dwgs:					
a) Plans	x				
b) Sections	х				
c) Elevations	х				
d) Other (describe) (maybe closed - may include a LOI for closing)	x				
A Design Information					
a) Name of Designer of Pecord	v				
a) Name of Designer of Record	^	v			
o) Design Assumptions		A V			
c) Design Analyses (including Hydrologic/Hydraulic & Slope Stability Analyses		x			



Available Information Checklist Coal Combustion Residuals Impoundmer	(Continued nt (CCRI) Dam	l)			
ITEM DESCRIPTION		PROVIDED BY UTILITY			
		ES	NO	N/A	
d) Spillway Design Flood or Design Basis				х	
e) Slope Stability Factors of Safety			х		
f) Design Soil Properties and Parameters		x			
g) Other (describe)					
5. Permits:					
a) NPDES? Number? TX00878	15 :	x			
b) Dam Safety - Operating Permit? Number? n/a				х	
c) Other (describe) WQ0002574000 - unknown	n permit no.				
6. Subsurface Information:					
a) Geology			х		
b) Geotechnical Report			х		
c) Test Boring Logs		х			
d) Subsurface Profiles			х		
f) Other (describe)					
7. Monitoring Information:					
a) Observation Wells/Piezometer Readings				х	
b) Seepage Readings				x	
c) Settlement Readings				x	
d) Alignment Readings				x	
e) Inclinometer Readings				x	
f) Time vs Reading Graphs				x	
g) Other (describe)					
8. Instrumentation Dwgs:					
a) Location Plan				x	
b) Section Views				x	
c) Other (describe)					



Available Information Checklist (Continued)

Coal Combustion Residuals Impoundment (CCRI) Dam

	PROVIDED BY UTILITY		
TIEM DESCRIPTION		NO	N/A
9. Operation, Maintenance, & Surveillance:			
a) Operating Procedures	x		
b) Maintenance Procedures	x		
c) Inspection Procedures	х		
d) Third Party Inspection Reports	х		
e) Other (describe)			
10. Miscellaneous:			
a) Construction Documentation/Foundation Prep	х		
b) Spills or Releases		x	
c) Repairs			x
d) Inundation Map		х	
e) Other (describe)			
f) Emergency Action Plan	х		

NOTE:

All data is to be furnished on CD to Dewberry by end of March 2011.



Page 3 of 3



Available Information Checklist - Addendum Coal Combustion Residuals Impoundment (CCRI) Dam					
		PROVIDED BY UTILITY			
TEW DESCRIPTION	YES	NO	N/A		
11. Coal Combustion Residuals Handling Equipment Train:					
a) Fly Ash (dry train - described below)					
Generation & Collection Methods; Equipment	х				
Transport to Storage Methods; Equipment; Containment Methods	х				
Storage Methods; Equipment; Containment Methods	х				
Transport to Disposition Methods; Equipment; Containment Methods	х				
b) Bottom Ash (wet train)					
Generation & Collection Methods; Equipment	х				
Transport to Storage Methods; Equipment; Containment Methods	х				
Storage Methods; Equipment; Containment Methods	х				
Transport to Disposition Methods; Equipment; Containment Methods	х				
c) Boiler Slag (same as Bottom Ash)					
Generation & Collection Methods; Equipment	х				
Transport to Storage Methods; Equipment; Containment Methods	х				
Storage Methods; Equipment; Containment Methods	х				
Transport to Disposition Methods; Equipment; Containment Methods	х				
d) Flue Gas Desulfurization Sludge (inspected)					
Generation & Collection Methods; Equipment	х				
Transport to Storage Methods; Equipment; Containment Methods	х				
Storage Methods; Equipment; Containment Methods	х				
Transport to Disposition Methods; Equipment; Containment Methods	х				
Note: AEP representatives preferred not to tour and inspect the Fly Ash o	lisposal	train.			
However they did describe the process below.					

Fly Ash Disposal Process (dry train):

- 1. Fly ash is electrostatically precipitated and conveyed by gravity to a hopper
- 2. The ash is then pneumatically conveyed to a holding silo
- 3. The ash is then loaded via gravity feed into trucks
- 4. The ash is stock piled there to be either sold to a 3rd party for beneficial reuse or to be permanently disposed of in the landfill

Page A-1



Appendix C – Additional Photographs



Figure C-1. Additional incised CCR pond #21



Figure C-2. Additional incised CCR pond #22



Figure C-3. Additional incised CCR pond #23



Figure C-4. Additional incised CCR pond #7